

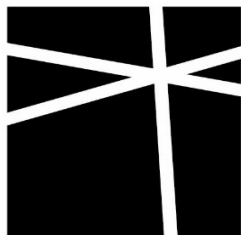
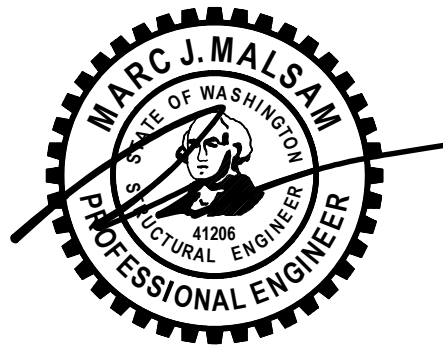
SUPPLEMENTAL STRUCTURAL CALCULATIONS FOR:

6221 83RD PL SE

MERCER ISLAND, WA

ARCHITECT: JULIAN WEBER ARCH + DESIGN

JANUARY 6, 2023



**MALSAM
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STRUCTURAL
ENGINEERING

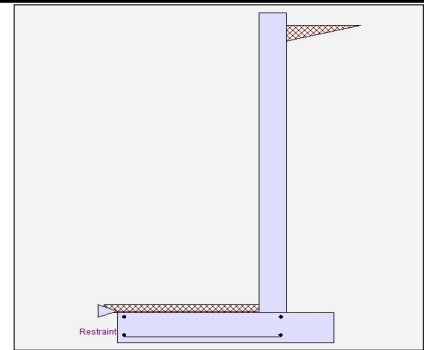
Cantilevered Retaining Wall

Criteria

Retained Height	=	12.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	108.0 #/ft
...Height to Top	=	12.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.42 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	6,105 lbs
...resultant ecc.	=	27.59 in

Soil Pressure @ Toe	=	2,393 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	3,350 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	51.4 psi OK
Footing Shear @ Heel	=	21.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	4,807.3 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	12.00
Rebar Size	=	# 6
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.997
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	5,904.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	26,208.0
Moment....Allowable	=	26,273.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	51.1
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	9.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	150.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.6258 in ² /ft		
(4/3) * As :	0.8344 in ² /ft	Min Stem T&S Reinf Area 3.600 in ²	
200bd/fy : 200(12)(9.625)/60000 :	0.385 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.288 in ² /ft	
0.0018bh : 0.0018(12)(12) :	0.2592 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.6258 in ² /ft	#4@ 8.33 in	#4@ 16.67 in
Provided Area :	0.66 in ² /ft	#5@ 12.92 in	#5@ 25.83 in
Maximum Area :	1.3039 in ² /ft	#6@ 18.33 in	#6@ 36.67 in

Footing Data

Toe Width	=	5.25 ft
Heel Width	=	2.75
Total Footing Width	=	8.00
Footing Thickness	=	15.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 3,350	0 psf
Mu' : Upward	= 364,019	0 ft-#
Mu' : Downward	= 45,147	2,991 ft-#
Mu: Design	= 26,573	2,991 ft-#
Actual 1-Way Shear	= 51.38	21.91 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 4.54 in, #5@ 7.04 in, #6@ 9.99 in, #7@ 13.63 in, #8@ 17.95 in, #9@ 22.7
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	2.59	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 7.41 in		#4@ 14.81 in
#5@ 11.48 in		#5@ 22.96 in
#6@ 16.30 in		#6@ 32.59 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	3,511.3	4.42	15,508.0	Soil Over HL (ab. water tbl)	2,520.0	7.13	17,955.0
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		7.13	17,955.0
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	1,296.0	7.25	9,396.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	210.0	2.63	551.3
				Surcharge Over Toe =			
				Stem Weight(s) =	1,875.0	5.75	10,781.3
				Earth @ Stem Transitions =			
Total	= 4,807.3	O.T.M. =	24,904.0	Footing Weight =	1,500.0	4.00	6,000.0
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.42		Total =	6,105.0 lbs	R.M.=	35,287.5
Vertical Loads used for Soil Pressure =		6,105.0 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.104 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall

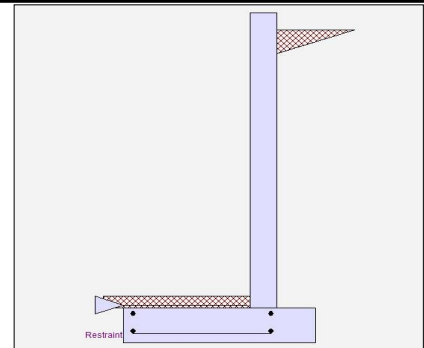
Code: IBC 2015,ACI 318-14,ACI 530-13

Criteria

Retained Height	=	8.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.91 OK
Slab Resists All Sliding !		

Total Bearing Load	=	2,670 lbs
...resultant ecc.	=	9.62 in

Soil Pressure @ Toe	=	1,077 psf OK
Soil Pressure @ Heel	=	11 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,507 psf
ACI Factored @ Heel	=	15 psf
Footing Shear @ Toe	=	21.9 psi OK
Footing Shear @ Heel	=	9.7 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,620.0 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	11.25
Rebar Placed at	=	6.25 i

Design Data

fb/FB + fa/Fa	=	0.626
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,048.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	5,461.3
Moment....Allowable	=	8,719.4

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	27.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.2046 in ² /ft		
(4/3) * As :	0.2728 in ² /ft	Min Stem T&S Reinf Area 1.632 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.25 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.3307 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	3.25 ft
Heel Width	=	1.66
Total Footing Width	=	4.91
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,507	15 psf
Mu' : Upward	= 74,669	57 ft-#
Mu' : Downward	= 14,450	657 ft-#
Mu: Design	= 5,018	600 ft-#
Actual 1-Way Shear	= 21.86	9.65 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	1.27	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,620.0	3.00	4,860.0	Soil Over HL (ab. water tbl)	953.6	4.41	4,208.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.41	4,208.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	130.0	1.63	211.3
				Surcharge Over Toe =			
				Stem Weight(s) =	850.0	3.58	3,045.8
				Earth @ Stem Transitions =			
Total	= 1,620.0	O.T.M.	= 4,860.0	Footing Weight =	736.5	2.46	1,808.1
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		=	1.91	Total =	2,670.1 lbs	R.M.=	9,273.7
Vertical Loads used for Soil Pressure =		2,670.1	lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.052 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

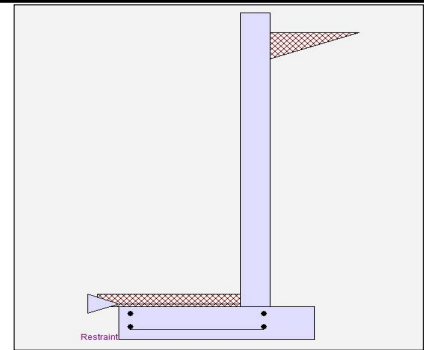
Cantilevered Retaining Wall

Criteria

Retained Height	=	7.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	63.0 #/ft
...Height to Top	=	7.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.37 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	2,255 lbs
...resultant ecc.	=	16.56 in

Soil Pressure @ Toe	=	1,812 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	2,536 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	33.9 psi OK
Footing Shear @ Heel	=	12.1 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,668.2 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	8.75
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.711
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,009.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	5,202.2
Moment....Allowable	=	7,314.8

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	26.8
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1949 in ² /ft		
(4/3) * As :	0.2599 in ² /ft	Min Stem T&S Reinf Area 1.440 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.25 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2743 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	2.75 ft
Heel Width	=	1.67
Total Footing Width	=	4.42
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,536	0 psf
Mu' : Upward	= 72,750	0 ft-#
Mu' : Downward	= 8,984	583 ft-#
Mu: Design	= 5,314	583 ft-#
Actual 1-Way Shear	= 33.90	12.10 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.42 in, #5@ 14.60 in, #6@ 20.72 in, #7@ 28.26 in, #8@ 37.21 in, #9@ 47
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	0.95	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,227.2	2.61	3,204.4	Soil Over HL (ab. water tbl)	842.8	3.92	3,302.4
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.92	3,302.4
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	441.0	4.33	1,911.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	110.0	1.38	151.3
				Surcharge Over Toe =			
				Stem Weight(s) =	750.0	3.08	2,312.5
				Earth @ Stem Transitions =			
Total	= 1,668.2	O.T.M. =	5,115.4	Footing Weight =	552.5	2.21	1,221.0
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.37		Total =	2,255.3 lbs	R.M.=	6,987.1
Vertical Loads used for Soil Pressure =		2,255.3 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.085 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

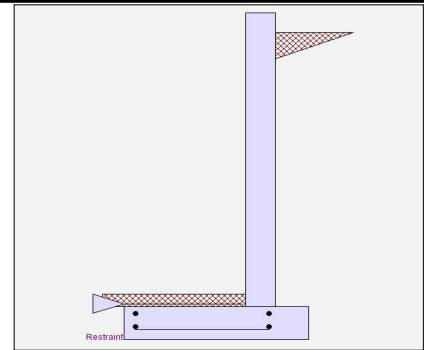
Cantilevered Retaining Wall

Criteria

Retained Height	=	7.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footings Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.86 OK
Slab Resists All Sliding !		

Total Bearing Load	=	2,014 lbs
...resultant ecc.	=	8.66 in

Soil Pressure @ Toe	=	985 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,378 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	22.4 psi OK
Footing Shear @ Heel	=	8.3 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,227.2 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	9.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.513
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,568.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	3,658.7
Moment....Allowable	=	7,122.4

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	20.9
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1371 in ² /ft		
(4/3) * As :	0.1828 in ² /ft	Min Stem T&S Reinf Area 1.440 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1828 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2667 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	2.75 ft
Heel Width	=	1.42
Total Footing Width	=	4.17
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,378	0 psf
Mu' : Upward	= 48,532	17 ft-#
Mu' : Downward	= 8,984	329 ft-#
Mu: Design	= 3,296	311 ft-#
Actual 1-Way Shear	= 22.43	8.29 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: No key defined

Min footing T&S reinf Area	0.90	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,227.2	2.61	3,204.4	Soil Over HL (ab. water tbl)	632.8	3.79	2,400.4
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.79	2,400.4
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	110.0	1.38	151.3
				Surcharge Over Toe =			
				Stem Weight(s) =	750.0	3.08	2,312.5
				Earth @ Stem Transitions =			
Total	= 1,227.2	O.T.M. =	3,204.4	Footing Weight =	521.3	2.09	1,086.8
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.86		Total =	2,014.1 lbs	R.M.=	5,951.0
Vertical Loads used for Soil Pressure =		2,014.1 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.049 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

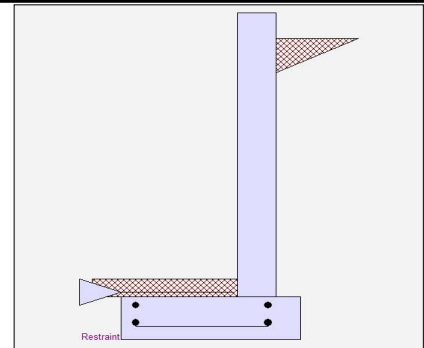
Cantilevered Retaining Wall

Criteria

Retained Height	=	5.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	45.0 #/ft
...Height to Top	=	5.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.29 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	1,263 lbs
...resultant ecc.	=	12.82 in

Soil Pressure @ Toe	=	1,786 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	2,500 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	18.1 psi OK
Footing Shear @ Heel	=	4.4 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	905.6 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	10.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.307
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,075.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	1,979.2
Moment....Allowable	=	6,444.1

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	14.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0742 in ² /ft		
(4/3) * As :	0.0989 in ² /ft	Min Stem T&S Reinf Area 1.056 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.24 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	2.00 ft
Heel Width	=	1.08
Total Footing Width	=	3.08
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,500	0 psf
Mu' : Upward	= 32,432	0 ft-#
Mu' : Downward	= 5,544	87 ft-#
Mu: Design	= 2,241	87 ft-#
Actual 1-Way Shear	= 18.13	4.37 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 10.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	0.67	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	680.6	1.94	1,323.3	Soil Over HL (ab. water tbl)	248.0	2.87	712.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.87	712.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	225.0	3.33	750.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	80.0	1.00	80.0
				Surcharge Over Toe =			
				Stem Weight(s) =	550.0	2.33	1,283.3
				Earth @ Stem Transitions =			
Total	= 905.6	O.T.M. =	2,073.3	Footing Weight =	385.0	1.54	592.9
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.29		Total =	1,263.0 lbs	R.M.=	2,668.8
Vertical Loads used for Soil Pressure =		1,263.0 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt**Horizontal Deflection at Top of Wall due to settlement of soil**

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.089 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

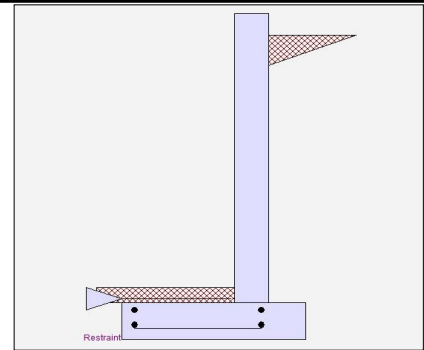
Cantilevered Retaining Wall

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footings Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.06 OK
Slab Resists All Sliding !		

Total Bearing Load	=	1,733 lbs
...resultant ecc.	=	6.37 in

Soil Pressure @ Toe	=	886 psf OK
Soil Pressure @ Heel	=	61 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,240 psf
ACI Factored @ Heel	=	86 psf
Footing Shear @ Toe	=	16.6 psi OK
Footing Shear @ Heel	=	6.3 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	933.9 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.391
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,152.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,304.0
Moment....Allowable	=	5,883.6

Shear....Actual

Service Level	psi =	
Strength Level	psi =	15.4
Shear....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0863 in ² /ft		
(4/3) * As :	0.1151 in ² /ft	Min Stem T&S Reinf Area 1.248 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2182 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	2.25 ft
Heel Width	=	1.41
Total Footing Width	=	3.66
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,240	86 psf
Mu' : Upward	= 30,479	45 ft-#
Mu' : Downward	= 6,014	280 ft-#
Mu: Design	= 2,039	235 ft-#
Actual 1-Way Shear	= 16.57	6.28 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: No key defined

Min footing T&S reinf Area	0.79	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	933.9	2.28	2,127.2	Soil Over HL (ab. water tbl)	535.2	3.29	1,759.9
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.29	1,759.9
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	90.0	1.13	101.3
=				Surcharge Over Toe =			
Total	933.9	O.T.M.	2,127.2	Stem Weight(s) =	650.0	2.58	1,679.2
				Earth @ Stem Transitions =			
				Footing Weight =	457.5	1.83	837.2
				Key Weight =			
				Vert. Component =			
				Total =	1,732.7	R.M.=	4,377.6

Resisting/Overturning Ratio = 2.06
 Vertical Loads used for Soil Pressure = 1,732.7 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.044 in

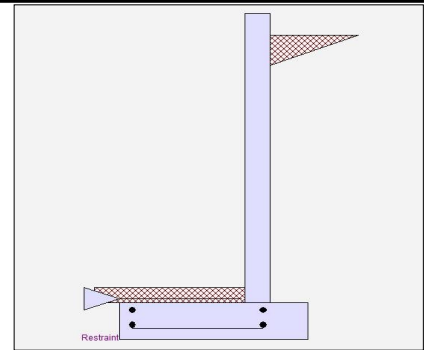
The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall**Criteria**

Retained Height	=	6.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	54.0 #/ft
...Height to Top	=	6.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.24 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	1,596 lbs
...resultant ecc.	=	16.50 in

Soil Pressure @ Toe	=	2,129 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	2,981 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	23.3 psi OK
Footing Shear @ Heel	=	7.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,257.9 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.835
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,476.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	3,276.0
Moment....Allowable	=	3,920.0

Shear....Actual

Service Level	psi =	
Strength Level	psi =	28.9
Shear....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1852 in2/ft		
(4/3) * As :	0.2469 in2/ft	Min Stem T&S Reinf Area 0.936 in2	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in2/ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1852 in2/ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2182 in2/ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in2/ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	2.50 ft
Heel Width	=	1.25
Total Footing Width	=	3.75
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,981	0 psf
Mu' : Upward	= 53,639	0 ft-#
Mu' : Downward	= 7,425	285 ft-#
Mu: Design	= 3,851	285 ft-#
Actual 1-Way Shear	= 23.28	7.92 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: No key defined

Min footing T&S reinf Area	0.81	in2
Min footing T&S reinf Area per foot	0.22	in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	933.9	2.28	2,127.2	Soil Over HL (ab. water tbl)	540.0	3.38	1,822.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.38	1,822.5
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	324.0	3.83	1,242.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	100.0	1.25	125.0
=				Surcharge Over Toe =			
Total =	1,257.9	O.T.M. =	3,369.2	Stem Weight(s) =	487.5	2.75	1,340.6
				Earth @ Stem Transitions =			
				Footing Weight =	468.8	1.88	878.9
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio =			1.24	Total =	1,596.3 lbs	R.M.=	4,167.0
Vertical Loads used for Soil Pressure =		1,596.3 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.103 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

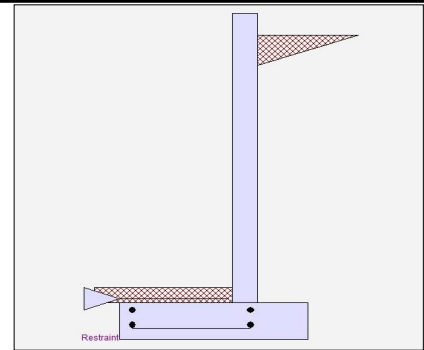
Cantilevered Retaining Wall

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footings Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.13 OK
Slab Resists All Sliding !		

Total Bearing Load	=	1,766 lbs
...resultant ecc.	=	6.11 in

Soil Pressure @ Toe	=	855 psf OK
Soil Pressure @ Heel	=	87 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,197 psf
ACI Factored @ Heel	=	122 psf
Footing Shear @ Toe	=	16.2 psi OK
Footing Shear @ Heel	=	7.8 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	933.9 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.587
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,152.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,304.0
Moment....Allowable	=	3,920.0

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	22.6
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1302 in ² /ft		
(4/3) * As :	0.1737 in ² /ft	Min Stem T&S Reinf Area 0.936 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.17 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2182 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	2.25	ft
Heel Width	=	1.50	
Total Footing Width	=	3.75	
Footing Thickness	=	10.00	in
Key Width	=	0.00	in
Key Depth	=	0.00	in
Key Distance from Toe	=	0.00	ft
f'c =	2,500	psi	Fy = 60,000
Footing Concrete Density	=	150.00	pcf
Min. As %	=	0.0018	
Cover @ Top	2.00	@ Btm.=	3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,197	122 psf
Mu' : Upward	= 29,827	109 ft-#
Mu' : Downward	= 6,014	507 ft-#
Mu: Design	= 1,984	398 ft-#
Actual 1-Way Shear	= 16.17	7.80 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	0.81	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	933.9	2.28	2,127.2	Soil Over HL (ab. water tbl)	720.0	3.25	2,340.0
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.25	2,340.0
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	90.0	1.13	101.3
=				Surcharge Over Toe =			
Total	933.9	O.T.M.	2,127.2	Stem Weight(s) =	487.5	2.50	1,218.8
				Earth @ Stem Transitions =			
				Footing Weight =	468.8	1.88	878.9
				Key Weight =			
				Vert. Component =			
				Total =	1,766.3	lbs	R.M.= 4,538.9

Resisting/Overturning Ratio = 2.13
 Vertical Loads used for Soil Pressure = 1,766.3 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.041 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

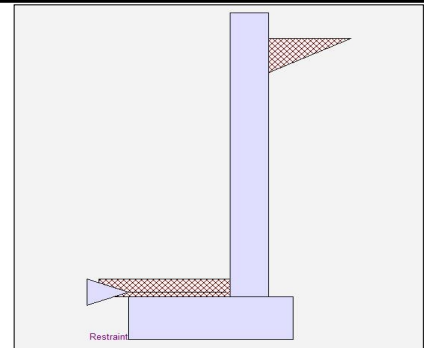
Cantilevered Retaining Wall

Criteria

Retained Height	=	5.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	250.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.78 OK
Slab Resists All Sliding !		

Total Bearing Load	=	1,222 lbs
...resultant ecc.	=	6.82 in

Soil Pressure @ Toe	=	962 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,346 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	10.2 psi OK
Footing Shear @ Heel	=	3.7 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	680.6 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.226
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	800.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	1,333.3
Moment....Allowable	=	5,883.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	10.7
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.05 in2/ft		
(4/3) * As :	0.0666 in2/ft	Min Stem T&S Reinf Area 1.056 in2	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2182 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	1.75 ft
Heel Width	=	1.08
Total Footing Width	=	2.83
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,346	0 psf
Mu' : Upward	= 19,058	0 ft-#
Mu' : Downward	= 3,638	74 ft-#
Mu: Design	= 1,285	74 ft-#
Actual 1-Way Shear	= 10.22	3.70 psi
Allow 1-Way Shear	= 40.00	40.00 psi
Toe Reinforcing	= None Spec'd	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: $\phi Mn = \phi^5 \lambda \sqrt{f_c} S_m$
 Heel: $\phi Mn = \phi^5 \lambda \sqrt{f_c} S_m$
 Key: No key defined

Min footing T&S reinf Area	0.61	in2
Min footing T&S reinf Area per foot	0.22	in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	680.6	1.94	1,323.3	Soil Over HL (ab. water tbl)	248.0	2.62	650.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.62	650.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	70.0	0.88	61.3
				Surcharge Over Toe =			
				Stem Weight(s) =	550.0	2.08	1,145.8
				Earth @ Stem Transitions =			
Total	= 680.6	O.T.M. =	1,323.3	Footing Weight =	353.8	1.42	500.6
				Key Weight =			
				Vert. Component =			
				Total =	1,221.8 lbs	R.M.=	2,358.2

Resisting/Overturning Ratio = 1.78
 Vertical Loads used for Soil Pressure = 1,221.8 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.052 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

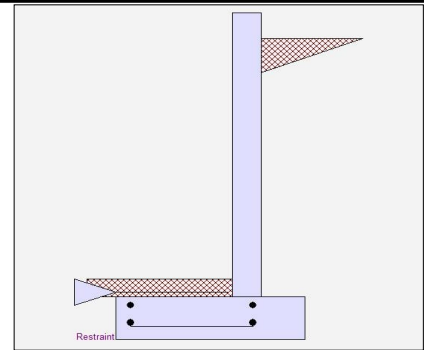
Cantilevered Retaining Wall

Criteria

Retained Height	=	5.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	250.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	45.0 #/ft
...Height to Top	=	5.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.43 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	1,349 lbs
...resultant ecc.	=	11.59 in

Soil Pressure @ Toe	=	1,365 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,910 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	18.5 psi OK
Footing Shear @ Heel	=	6.8 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	905.6 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.483
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,025.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	1,895.8
Moment....Allowable	=	3,920.0

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	20.1
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1072 in2/ft		
(4/3) * As :	0.1429 in2/ft	Min Stem T&S Reinf Area 0.792 in2	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in2/ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1429 in2/ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2182 in2/ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in2/ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	2.00 ft
Heel Width	=	1.25
Total Footing Width	=	3.25
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,910	0 psf
Mu' : Upward	= 30,387	0 ft-#
Mu' : Downward	= 4,752	245 ft-#
Mu: Design	= 2,136	245 ft-#
Actual 1-Way Shear	= 18.54	6.80 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: No key defined

Min footing T&S reinf Area	0.70	in2
Min footing T&S reinf Area per foot	0.22	in2 /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	680.6	1.94	1,323.3	Soil Over HL (ab. water tbl)	450.0	2.88	1,293.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.88	1,293.8
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	225.0	3.33	750.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	80.0	1.00	80.0
				Surcharge Over Toe =			
				Stem Weight(s) =	412.5	2.25	928.1
				Earth @ Stem Transitions =			
Total	= 905.6	O.T.M. =	2,073.3	Footing Weight =	406.3	1.63	660.2
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.43		Total =	1,348.8 lbs	R.M.=	2,962.0
Vertical Loads used for Soil Pressure =		1,348.8 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.064 in

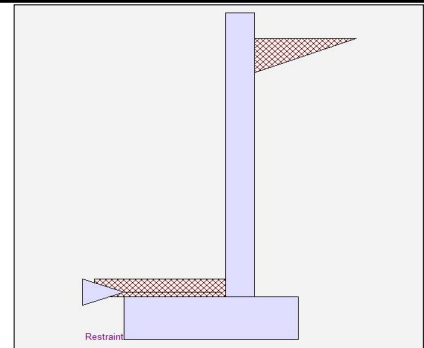
The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall**Criteria**

Retained Height	=	5.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footings Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.99 OK
Slab Resists All Sliding !		

Total Bearing Load	=	1,308 lbs
...resultant ecc.	=	6.01 in

Soil Pressure @ Toe	=	872 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,221 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	9.6 psi OK
Footing Shear @ Heel	=	5.6 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	680.6 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.369
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	800.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	1,333.3
Moment....Allowable	=	3,612.6

Shear....Actual

Service Level	psi =	
Strength Level	psi =	15.7
Shear....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0754 in ² /ft		
(4/3) * As :	0.1005 in ² /ft	Min Stem T&S Reinf Area 0.792 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1296 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	1.75 ft
Heel Width	=	1.25
Total Footing Width	=	3.00
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,221	0 psf
Mu' : Upward	= 18,072	28 ft-#
Mu' : Downward	= 3,638	245 ft-#
Mu: Design	= 1,203	216 ft-#
Actual 1-Way Shear	= 9.58	5.61 psi
Allow 1-Way Shear	= 40.00	40.00 psi
Toe Reinforcing	= None Spec'd	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: $\phi Mn = \phi'5\lambda\sqrt{fc}'Sm$
 Heel: $\phi Mn = \phi'5\lambda\sqrt{fc}'Sm$
 Key: No key defined

Min footing T&S reinf Area	0.65	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	680.6	1.94	1,323.3	Soil Over HL (ab. water tbl)	450.0	2.63	1,181.3
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.63	1,181.3
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	70.0	0.88	61.3
				Surcharge Over Toe =			
				Stem Weight(s) =	412.5	2.00	825.0
				Earth @ Stem Transitions =			
Total	= 680.6	O.T.M. =	1,323.3	Footing Weight =	375.0	1.50	562.5
				Key Weight =			
				Vert. Component =			
				Total =	1,307.5 lbs	R.M.=	2,630.0

Resisting/Overturning Ratio = 1.99
 Vertical Loads used for Soil Pressure = 1,307.5 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.044 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

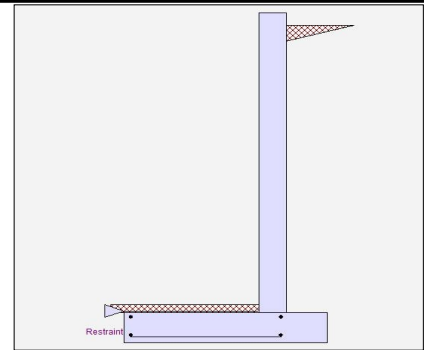
Cantilevered Retaining Wall

Criteria

Retained Height	=	12.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.98 OK
Slab Resists All Sliding !		

Total Bearing Load	=	5,641 lbs
...resultant ecc.	=	12.76 in

Soil Pressure @ Toe	=	1,392 psf OK
Soil Pressure @ Heel	=	113 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,949 psf
ACI Factored @ Heel	=	158 psf
Footing Shear @ Toe	=	34.5 psi OK
Footing Shear @ Heel	=	15.5 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	3,511.3 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	12.00
Rebar Size	=	# 6
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.701
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	4,608.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	18,432.0
Moment....Allowable	=	26,273.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	39.9
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	9.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	150.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.4401 in ² /ft		
(4/3) * As :	0.5868 in ² /ft	Min Stem T&S Reinf Area 3.600 in ²	
200bd/fy : 200(12)(9.625)/60000 :	0.385 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.288 in ² /ft	
0.0018bh : 0.0018(12)(12) :	0.2592 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.4401 in ² /ft	#4@ 8.33 in	#4@ 16.67 in
Provided Area :	0.66 in ² /ft	#5@ 12.92 in	#5@ 25.83 in
Maximum Area :	1.3039 in ² /ft	#6@ 18.33 in	#6@ 36.67 in

Footing Data

Toe Width	=	5.00 ft
Heel Width	=	2.50
Total Footing Width	=	7.50
Footing Thickness	=	15.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,949	158 psf
Mu' : Upward	= 232,581	312 ft-#
Mu' : Downward	= 40,950	2,197 ft-#
Mu: Design	= 15,969	1,886 ft-#
Actual 1-Way Shear	= 34.47	15.54 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.67 in, #5@ 8.79 in, #6@ 12.47 in, #7@ 17.01 in, #8@ 22.40 in, #9@ 28.
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	2.43	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 7.41 in	#4@ 14.81 in	
#5@ 11.48 in	#5@ 22.96 in	
#6@ 16.30 in	#6@ 32.59 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	3,511.3	4.42	15,508.0	Soil Over HL (ab. water tbl)	2,160.0	6.75	14,580.0
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		6.75	14,580.0
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	200.0	2.50	500.0
				Surcharge Over Toe =			
				Stem Weight(s) =	1,875.0	5.50	10,312.5
				Earth @ Stem Transitions =			
				Footing Weight =	1,406.3	3.75	5,273.4
				Key Weight =			
				Vert. Component =			
Total	= 3,511.3	O.T.M. =	15,508.0				
Resisting/Overturning Ratio		=	1.98				
Vertical Loads used for Soil Pressure =		5,641.3 lbs					
				Total =	5,641.3 lbs	R.M.=	30,665.9

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.064 in

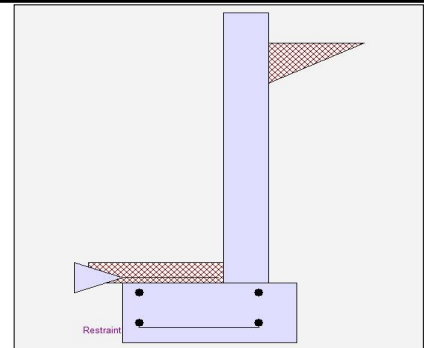
The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall**Criteria**

Retained Height	=	4.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	36.0 #/ft
...Height to Top	=	4.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.45 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	1,095 lbs
...resultant ecc.	=	9.18 in

Soil Pressure @ Toe	=	1,392 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,948 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	9.4 psi OK
Footing Shear @ Heel	=	2.6 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	644.0 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	9.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.136
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	656.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	970.7
Moment....Allowable	=	7,122.4

Shear....Actual

Service Level	psi =	
Strength Level	psi =	8.7
Shear....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0364 in ² /ft		
(4/3) * As :	0.0485 in ² /ft	Min Stem T&S Reinf Area 0.864 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2667 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	1.50 ft
Heel Width	=	1.08
Total Footing Width	=	2.58
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,948	0 psf
Mu' : Upward	= 17,949	0 ft-#
Mu' : Downward	= 3,078	65 ft-#
Mu: Design	= 1,239	65 ft-#
Actual 1-Way Shear	= 9.43	2.60 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	0.67	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	500.0	1.67	833.3	Soil Over HL (ab. water tbl)	198.4	2.37	470.9
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.37	470.9
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	144.0	3.00	432.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	60.0	0.75	45.0
				Surcharge Over Toe =			
				Stem Weight(s) =	450.0	1.83	825.0
				Earth @ Stem Transitions =			
Total	= 644.0	O.T.M. =	1,265.3	Footing Weight =	387.0	1.29	499.2
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.45		Total =	1,095.4 lbs	R.M.=	1,840.1
Vertical Loads used for Soil Pressure =		1,095.4 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.067 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

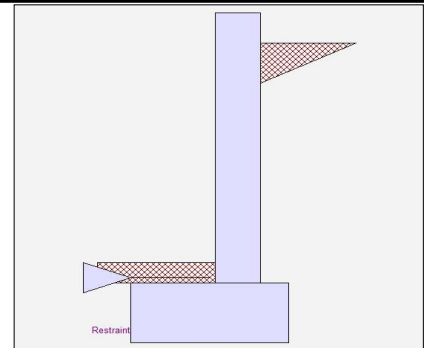
Cantilevered Retaining Wall

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.89 OK
Slab Resists All Sliding !		

Total Bearing Load	=	1,048 lbs
...resultant ecc.	=	5.52 in

Soil Pressure @ Toe	=	991 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,387 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	4.4 psi OK
Footing Shear @ Heel	=	2.5 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	500.0 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	9.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.095
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	512.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	682.7
Moment....Allowable	=	7,122.4

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	6.8
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0256 in ² /ft		
(4/3) * As :	0.0341 in ² /ft	Min Stem T&S Reinf Area 0.864 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2667 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	1.25 ft
Heel Width	=	1.08
Total Footing Width	=	2.33
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,387	0 psf
Mu' : Upward	= 10,443	1 ft-#
Mu' : Downward	= 2,138	65 ft-#
Mu: Design	= 692	64 ft-#
Actual 1-Way Shear	= 4.36	2.50 psi
Allow 1-Way Shear	= 40.00	40.00 psi
Toe Reinforcing	= None Spec'd	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: $\phi M_n = \phi'5' \lambda \sqrt{f_c} S_m$
 Heel: $\phi M_n = \phi'5' \lambda \sqrt{f_c} S_m$
 Key: No key defined

Min footing T&S reinf Area	0.60	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	500.0	1.67	833.3	Soil Over HL (ab. water tbl)	198.4	2.12	421.3
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.12	421.3
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	50.0	0.63	31.3
				Surcharge Over Toe =			
				Stem Weight(s) =	450.0	1.58	712.5
				Earth @ Stem Transitions =			
Total	= 500.0	O.T.M. =	833.3	Footing Weight =	349.5	1.17	407.2
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.89		Total =	1,047.9 lbs	R.M.=	1,572.2
Vertical Loads used for Soil Pressure =		1,047.9 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.053 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

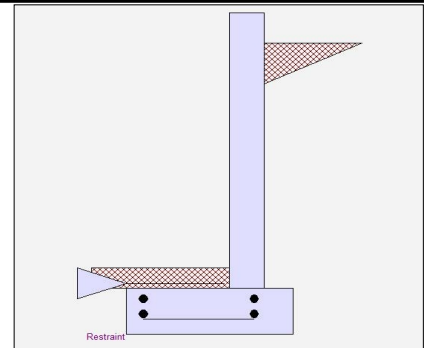
Cantilevered Retaining Wall

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	250.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	36.0 #/ft
...Height to Top	=	4.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.27 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	871 lbs
...resultant ecc.	=	10.39 in

Soil Pressure @ Toe	=	1,687 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	2,361 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	15.3 psi OK
Footing Shear @ Heel	=	3.6 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	595.3 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.268
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	656.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	970.7
Moment....Allowable	=	3,612.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	12.9
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0549 in ² /ft		
(4/3) * As :	0.0732 in ² /ft	Min Stem T&S Reinf Area 0.648 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1296 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	1.50 ft
Heel Width	=	0.92
Total Footing Width	=	2.42
Footing Thickness	=	9.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,361	0 psf
Mu' : Upward	= 16,916	0 ft-#
Mu' : Downward	= 2,471	63 ft-#
Mu: Design	= 1,204	63 ft-#
Actual 1-Way Shear	= 15.34	3.56 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 12.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 12.34 in, #5@ 19.13 in, #6@ 27.16 in, #7@ 37.03 in, #8@ 48.76 in, #9@ 6
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	0.47	in ²
Min footing T&S reinf Area per foot	0.19	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 12.35 in	#4@ 24.69 in	
#5@ 19.14 in	#5@ 38.27 in	
#6@ 27.16 in	#6@ 54.32 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	451.3	1.58	714.5	Soil Over HL (ab. water tbl)	201.6	2.21	445.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.21	445.5
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	144.0	2.75	396.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	60.0	0.75	45.0
=				Surcharge Over Toe =			
Total	= 595.3	O.T.M. =	1,110.5	Stem Weight(s) =	337.5	1.75	590.6
				Earth @ Stem Transitions =			
				Footing Weight =	272.3	1.21	329.4
				Key Weight =			
				Vert. Component =			
				Total =	871.4 lbs	R.M.=	1,410.6

Resisting/Overturning Ratio = 1.27
 Vertical Loads used for Soil Pressure = 871.4 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.087 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

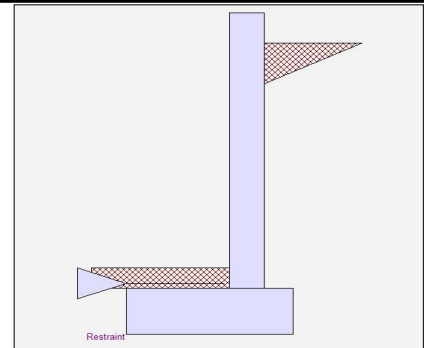
Cantilevered Retaining Wall

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.97 OK
Slab Resists All Sliding !		

Total Bearing Load	=	871 lbs
...resultant ecc.	=	4.93 in

Soil Pressure @ Toe	=	727 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,018 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	7.4 psi OK
Footing Shear @ Heel	=	3.2 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	451.3 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.188
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	512.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	682.7
Moment....Allowable	=	3,612.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	10.0
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0386 in2/ft		
(4/3) * As :	0.0515 in2/ft	Min Stem T&S Reinf Area 0.648 in2	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in2/ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1296 in2/ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2 in2/ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in2/ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	1.50 ft
Heel Width	=	0.92
Total Footing Width	=	2.42
Footing Thickness	=	9.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,018	0 psf
Mu' : Upward	= 10,876	4 ft-#
Mu' : Downward	= 2,471	63 ft-#
Mu: Design	= 700	58 ft-#
Actual 1-Way Shear	= 7.41	3.16 psi
Allow 1-Way Shear	= 40.00	40.00 psi
Toe Reinforcing	= None Spec'd	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: $\phi M_n = \phi'5' \lambda \sqrt{f_c} S_m$
 Heel: $\phi M_n = \phi'5' \lambda \sqrt{f_c} S_m$
 Key: No key defined

Min footing T&S reinf Area	0.47	in2
Min footing T&S reinf Area per foot	0.19	in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 12.35 in	#4@ 24.69 in	
#5@ 19.14 in	#5@ 38.27 in	
#6@ 27.16 in	#6@ 54.32 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	451.3	1.58	714.5	Soil Over HL (ab. water tbl)	201.6	2.21	445.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.21	445.5
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	60.0	0.75	45.0
				Surcharge Over Toe =			
				Stem Weight(s) =	337.5	1.75	590.6
				Earth @ Stem Transitions =			
				Footing Weight =	272.3	1.21	329.4
				Key Weight =			
				Vert. Component =			
Total	= 451.3	O.T.M. =	714.5	Total =	871.4 lbs	R.M.=	1,410.6
Resisting/Overturning Ratio		=	1.97				
Vertical Loads used for Soil Pressure =			871.4 lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.038 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

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Cantilevered Retaining Wall

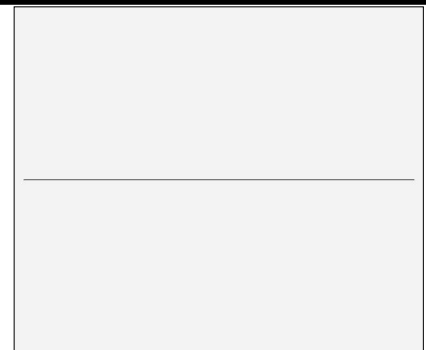
Code: IBC 2018,ACI 318-14,TMS 402-16

Criteria

Retained Height	=	0.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	0.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	0.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	0.0 psf/ft
Passive Pressure	=	0.0 psf/ft
Soil Density, Heel	=	0.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.000
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	(Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	0.00 OK
Sliding	=	0.00 OK
Total Bearing Load	=	0 lbs
...resultant ecc.	=	3.00 in
Soil Pressure @ Toe	=	0 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	0 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	0 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	25.9 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	67.1 psi
Sliding Calcs		
Lateral Sliding Force	=	0.0 lbs
less 0 % Passive Force	= -	0.0 lbs
less 0 % Friction Force	= -	0.0 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

Design Height Above Ftc	ft =	0.00
Wall Material Above "Ht"	=	3
Design Method	=	LRFD
Thickness	=	0.00
Rebar Size	=	# 0
Rebar Spacing	=	0.00
Rebar Placed at	=	Center

Design Data

fb/FB + fa/Fa	=	0.000
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	
Moment....Allowable	=	0.0

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	
Shear.....Allowable	psi =	0.0

Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	0.00

Masonry Data

f'm	psi =	1,500
Fs	psi =	32,000
Solid Grouting	=	No
Modular Ratio 'n'	=	0.00
Wall Weight	psf =	0.0
Short Term Factor	=	1.000
Equiv. Solid Thick.	in =	0.00
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	0.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.000
Seismic, E	1.700

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Cantilevered Retaining Wall

Code: IBC 2018,ACI 318-14,TMS 402-16

Footing Data

Toe Width	=	0.00 ft
Heel Width	=	0.50
Total Footing Width	=	0.50
Footing Thickness	=	0.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,000 psi	Fy = 60,000 psi
Footing Concrete Density	=	0.00 pcf
Min. As %	=	0.0000
Cover @ Top	0.00	@ Btm.= 0.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 0	0 psf
Mu' : Upward	= 0	0 ft-#
Mu' : Downward	= 0	0 ft-#
Mu: Design	= 0	0 ft-#
Actual 1-Way Shear	= 25.94	0.00 psi
Allow 1-Way Shear	= 35.78	35.78 psi
Toe Reinforcing	= None Spec'd	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: $\phi Mn = \phi'5' \lambda \sqrt{f'c} S_m$
 Heel: $\phi Mn = \phi'5' \lambda \sqrt{f'c} S_m$
 Key: No key defined

Min footing T&S reinf Area 0.00 in2
 Min footing T&S reinf Area per foot 0.00 in2 /ft
 If one layer of horizontal bars: #4 @ 0.00 in
 #5 @ 0.00 in
 #6 @ 0.00 in
 If two layers of horizontal bars:
 #4 @ 0.00 in
 #5 @ 0.00 in
 #6 @ 0.00 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING....		RESISTING....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)				Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		
Hydrostatic Force				Watre Table		
Buoyant Force	=			Sloped Soil Over Heel	=	
Surcharge over Heel	=			Surcharge Over Heel	=	
Surcharge Over Toe	=			Adjacent Footing Load	=	
Adjacent Footing Load	=			Axial Dead Load on Stem	=	
Added Lateral Load	=			* Axial Live Load on Stem	=	
Load @ Stem Above Soil	=			Soil Over Toe	=	
	=			Surcharge Over Toe	=	
				Stem Weight(s)	=	
				Earth @ Stem Transitions	=	
Total	=	O.T.M.	=	Footing Weight	=	0.25
				Key Weight	=	
				Vert. Component	=	
Resisting/Overturning Ratio		=		Total	=	lbs R.M.=
Vertical Loads used for Soil Pressure	=		lbs			

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Cantilevered Retaining Wall

Criteria

Retained Height	=	0.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	0.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	0.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	0.0 psf/ft
	=	
Passive Pressure	=	0.0 psf/ft
Soil Density, Heel	=	0.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.000
Soil height to ignore for passive pressure	=	0.00 in

Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	
		(Service Level)
Wind on Exposed Stem	=	0.0 psf
(Service Level)		

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.000

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	0.00 OK
Sliding	=	0.00 OK
Total Bearing Load	=	0 lbs
...resultant ecc.	=	0.00 in
Soil Pressure @ Toe	=	0 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	0 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	0 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	25.9 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	67.1 psi
Sliding Calcs		
Lateral Sliding Force	=	0.0 lbs
less 0 % Passive Force	= -	0.0 lbs
less 0 % Friction Force	= -	0.0 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS
NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.000
Seismic, E	1.700

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	3
Design Method	=	LRFD
Thickness	=	0.00
Rebar Size	=	# 0
Rebar Spacing	=	0.00
Rebar Placed at	=	Center

Design Data

fb/FB + fa/Fa	=	0.000
---------------	---	-------

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	
Moment....Allowable	=	0.0

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	
Shear.....Allowable	psi =	0.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	0.00

Masonry Data

f'm	psi =	1,500
Fs	psi =	0
Solid Grouting	=	No
Modular Ratio 'n'	=	0.00
Wall Weight	psf =	0.0
Short Term Factor	=	1.000
Equiv. Solid Thick.	in =	0.00
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	0.0

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Cantilevered Retaining Wall

Code: IBC 2018,ACI 318-14,TMS 402-16

Footing Data

Toe Width = 0.00 ft
 Heel Width = 0.00
 Total Footing Width = 0.00
 Footing Thickness = 0.00 in
 Key Width = 0.00 in
 Key Depth = 0.00 in
 Key Distance from Toe = 0.00 ft
 f'c = 2,000 psi Fy = 60,000 psi
 Footing Concrete Density = 0.00 pcf
 Min. As % = 0.0000
 Cover @ Top 0.00 @ Btm.= 0.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure =	0	0 psf
Mu' : Upward =	0	0 ft-#
Mu' : Downward =	0	0 ft-#
Mu: Design =	0	0 ft-#
Actual 1-Way Shear =	25.94	0.00 psi
Allow 1-Way Shear =	35.78	35.78 psi
Toe Reinforcing =	None Spec'd	
Heel Reinforcing =	None Spec'd	
Key Reinforcing =	None Spec'd	
Footing Torsion, Tu =	0.00 ft-lbs	
Footing Allow. Torsion, phi Tu =	0.00 ft-lbs	

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: $\phi M_n = \phi'5' \lambda \sqrt{f_c} S_m$
 Heel: $\phi M_n = \phi'5' \lambda \sqrt{f_c} S_m$
 Key: No key defined

Min footing T&S reinf Area 0.00 in²
 Min footing T&S reinf Area per foot 0.00 in² /ft
 If one layer of horizontal bars: If two layers of horizontal bars:
 #4@ 0.00 in #4@ 0.00 in
 #5@ 0.00 in #5@ 0.00 in
 #6@ 0.00 in #6@ 0.00 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING....		RESISTING....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)				Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		
Hydrostatic Force				Watre Table		
Buoyant Force =				Sloped Soil Over Heel =		
Surcharge over Heel =				Surcharge Over Heel =		
Surcharge Over Toe =				Adjacent Footing Load =		
Adjacent Footing Load =				Axial Dead Load on Stem =		
Added Lateral Load =				* Axial Live Load on Stem =		
Load @ Stem Above Soil =				Soil Over Toe =		
=				Surcharge Over Toe =		
				Stem Weight(s) =		
				Earth @ Stem Transitions=		
				Footing Weight =		
				Key Weight =		
				Vert. Component =		
Total	=	O.T.M.	=	Total	=	lbs R.M.=
Resisting/Overturning Ratio		=				
Vertical Loads used for Soil Pressure =			lbs			

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

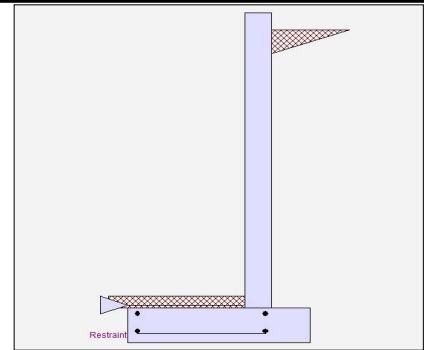
Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Cantilevered Retaining Wall**Criteria**

Retained Height	=	8.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
Passive Pressure	=	250.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	64.0 #/ft
...Height to Top	=	8.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	(Service Level)
Wind on Exposed Stem	=	0.0 psf
(Service Level)		

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.26 Ratio < 1.5!
Slab Resists All Sliding!		

Total Bearing Load	=	2,623 lbs
...resultant ecc.	=	19.73 in

Soil Pressure @ Toe	=	2,548 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	3,567 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	29.8 psi OK
Footing Shear @ Heel	=	12.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,929.5 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.877
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,416.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	7,125.3
Moment....Allowable	=	8,121.3

Shear....Actual

Service Level	psi =	
Strength Level	psi =	32.5
Shear....Allowable	psi =	75.0
Anet (Masonry)	in ² =	
Rebar Depth 'd'	in =	6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.2698 in2/ft		
(4/3) * As :	0.3597 in2/ft	Min Stem T&S Reinf Area 0.000 in2	
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.000 in2/ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.2698 in2/ft	#4@ 0.00 in	#4@ 0.00 in
Provided Area :	0.31 in2/ft	#5@ 0.00 in	#5@ 0.00 in
Maximum Area :	0.8382 in2/ft	#6@ 0.00 in	#6@ 0.00 in

Footing Data

Toe Width	=	3.00 ft
Heel Width	=	1.66
Total Footing Width	=	4.66
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 3,567	0 psf
Mu' : Upward	= 101,945	0 ft-#
Mu' : Downward	= 14,364	767 ft-#
Mu: Design	= 7,298	767 ft-#
Actual 1-Way Shear	= 29.75	12.86 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.07 in, #5@ 14.07 in, #6@ 19.97 in, #7@ 27.23 in, #8@ 35.86 in, #9@ 45
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: No key defined

Min footing T&S reinf Area	0.00	in2
Min footing T&S reinf Area per foot	0.00	in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 0.00 in	#4@ 0.00 in	
#5@ 0.00 in	#5@ 0.00 in	
#6@ 0.00 in	#6@ 0.00 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,417.5	3.00	4,252.5	Soil Over HL (ab. water tbl)	953.6	4.16	3,970.2
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.16	3,970.2
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	512.0	5.00	2,560.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	120.0	1.50	180.0
				Surcharge Over Toe =			
				Stem Weight(s) =	850.0	3.33	2,833.3
				Earth @ Stem Transitions =			
Total	= 1,929.5	O.T.M. =	6,812.5	Footing Weight =	699.0	2.33	1,628.7
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.26		Total =	2,622.6 lbs	R.M.=	8,612.2
Vertical Loads used for Soil Pressure =		2,622.6 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.129 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

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Cantilevered Retaining Wall

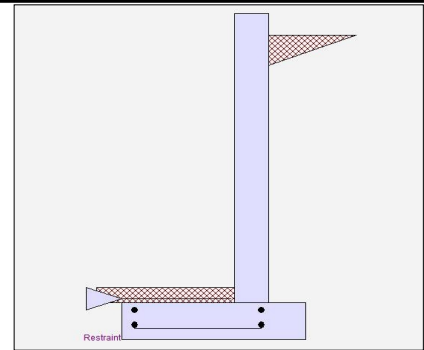
Code: IBC 2015,ACI 318-14,ACI 530-13

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	54.0 #/ft
...Height to Top	=	6.00 ft
...Height to Bottom	=	0.10 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.30 Ratio < 1.5!
Slab Resists All Sliding !		
Total Bearing Load	=	1,733 lbs
...resultant ecc.	=	15.01 in
Soil Pressure @ Toe	=	1,993 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,791 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	26.4 psi OK
Footing Shear @ Heel	=	7.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,263.3 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.556
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,476.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	3,276.0
Moment....Allowable	=	5,883.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	19.7
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1227 in ² /ft		
(4/3) * As :	0.1637 in ² /ft	Min Stem T&S Reinf Area 1.248 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2182 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	2.25 ft
Heel Width	=	1.41
Total Footing Width	=	3.66
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,791	0 psf
Mu' : Upward	= 48,627	0 ft-#
Mu' : Downward	= 6,014	280 ft-#
Mu: Design	= 3,551	280 ft-#
Actual 1-Way Shear	= 26.40	7.85 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	0.79	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	933.9	2.28	2,127.2	Soil Over HL (ab. water tbl)	535.2	3.29	1,759.9
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.29	1,759.9
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	329.4	3.78	1,246.2	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	90.0	1.13	101.3
				Surcharge Over Toe =			
				Stem Weight(s) =	650.0	2.58	1,679.2
				Earth @ Stem Transitions =			
Total	= 1,263.3	O.T.M.	= 3,373.4	Footing Weight =	457.5	1.83	837.2
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		=	1.30	Total =	1,732.7 lbs	R.M.=	4,377.6
Vertical Loads used for Soil Pressure =		1,732.7	lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt**Horizontal Deflection at Top of Wall due to settlement of soil**

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.098 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

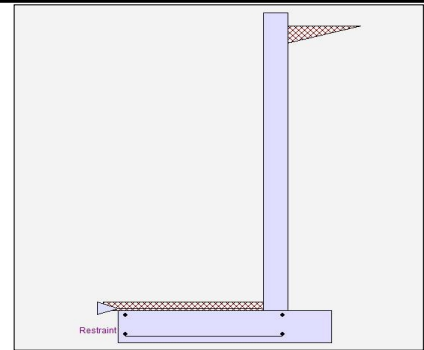
Cantilevered Retaining Wall

Criteria

Retained Height	=	11.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footings Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	99.0 #/ft
...Height to Top	=	11.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.34 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	4,987 lbs
...resultant ecc.	=	27.81 in

Soil Pressure @ Toe	=	2,468 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	3,455 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	42.1 psi OK
Footing Shear @ Heel	=	17.4 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	4,090.3 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 6
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.992
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	4,961.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	20,186.8
Moment....Allowable	=	20,333.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	54.2
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	7.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	125.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.614 in ² /ft		
(4/3) * As :	0.8187 in ² /ft	Min Stem T&S Reinf Area 2.760 in ²	
200bd/fy : 200(12)(7.625)/60000 :	0.305 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft	
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.614 in ² /ft	#4@ 10.00 in	#4@ 20.00 in
Provided Area :	0.66 in ² /ft	#5@ 15.50 in	#5@ 31.00 in
Maximum Area :	1.0329 in ² /ft	#6@ 22.00 in	#6@ 44.00 in

Footing Data

Toe Width	=	5.00 ft
Heel Width	=	2.33
Total Footing Width	=	7.33
Footing Thickness	=	15.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 3,455	0 psf
Mu' : Upward	= 306,045	0 ft-#
Mu' : Downward	= 40,950	2,026 ft-#
Mu: Design	= 22,091	2,026 ft-#
Actual 1-Way Shear	= 42.13	17.36 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.21 in, #5@ 8.08 in, #6@ 11.47 in, #7@ 15.65 in, #8@ 20.60 in, #9@ 26.
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	2.37	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 7.41 in	#4@ 14.81 in	
#5@ 11.48 in	#5@ 22.96 in	
#6@ 16.30 in	#6@ 32.59 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	3,001.3	4.08	12,255.1	Soil Over HL (ab. water tbl)	1,975.6	6.58	13,002.7
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		6.58	13,002.7
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	1,089.0	6.75	7,350.8	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	200.0	2.50	500.0
=				Surcharge Over Toe =			
Total =	4,090.3	O.T.M. =	19,605.9	Stem Weight(s) =	1,437.5	5.42	7,786.5
				Earth @ Stem Transitions =			
				Footing Weight =	1,374.4	3.67	5,037.1
				Key Weight =			
				Vert. Component =			
				Total =	4,987.5 lbs	R.M.=	26,326.3

Resisting/Overturning Ratio = 1.34
 Vertical Loads used for Soil Pressure = 4,987.5 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.108 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

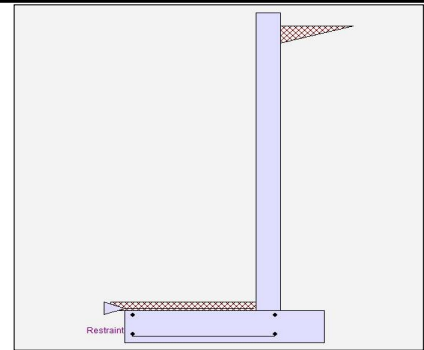
Cantilevered Retaining Wall

Criteria

Retained Height	=	11.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footings Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.95 OK
Slab Resists All Sliding !		

Total Bearing Load	=	4,874 lbs
...resultant ecc.	=	12.40 in

Soil Pressure @ Toe	=	1,362 psf OK
Soil Pressure @ Heel	=	66 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,906 psf
ACI Factored @ Heel	=	92 psf
Footing Shear @ Toe	=	29.4 psi OK
Footing Shear @ Heel	=	14.6 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	3,001.3 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 6
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.698
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	3,872.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	14,197.3
Moment....Allowable	=	20,333.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	42.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	7.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	125.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.4318 in ² /ft		
(4/3) * As :	0.5758 in ² /ft	Min Stem T&S Reinf Area 2.760 in ²	
200bd/fy : 200(12)(7.625)/60000 :	0.305 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft	
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.4318 in ² /ft	#4@ 10.00 in	#4@ 20.00 in
Provided Area :	0.66 in ² /ft	#5@ 15.50 in	#5@ 31.00 in
Maximum Area :	1.0329 in ² /ft	#6@ 22.00 in	#6@ 44.00 in

Footing Data

Toe Width	=	4.50 ft
Heel Width	=	2.33
Total Footing Width	=	6.83
Footing Thickness	=	15.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,906	92 psf
Mu' : Upward	= 183,187	251 ft-#
Mu' : Downward	= 33,170	2,026 ft-#
Mu: Design	= 12,501	1,775 ft-#
Actual 1-Way Shear	= 29.43	14.57 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 7.24 in, #5@ 11.22 in, #6@ 15.93 in, #7@ 21.73 in, #8@ 28.61 in, #9@ 36
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	2.21	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 7.41 in	#4@ 14.81 in	
#5@ 11.48 in	#5@ 22.96 in	
#6@ 16.30 in	#6@ 32.59 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	3,001.3	4.08	12,255.1	Soil Over HL (ab. water tbl)	1,975.6	6.08	12,014.9
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		6.08	12,014.9
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	180.0	2.25	405.0
=				Surcharge Over Toe =			
Total	3,001.3	O.T.M.	12,255.1	Stem Weight(s) =	1,437.5	4.92	7,067.7
				Earth @ Stem Transitions =			
				Footing Weight =	1,280.6	3.42	4,373.3
				Key Weight =			
				Vert. Component =			
				Total =	4,873.7 lbs	R.M.=	23,861.0

Resisting/Overturning Ratio = 1.95
 Vertical Loads used for Soil Pressure = 4,873.7 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.064 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Concrete Stem Rebar Area Details

	Vertical Reinforcing	Horizontal Reinforcing
Bottom Stem		
As (based on applied moment) :	0.4283 in ² /ft	
(4/3) * As :	0.5711 in ² /ft	Min Stem T&S Reinf Area 2.520 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.4283 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.465 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	4.25 ft
Heel Width	=	2.33
Total Footing Width	=	6.58
Footing Thickness	=	15.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 3,220	0 psf
Mu' : Upward	= 223,025	0 ft-#
Mu' : Downward	= 29,586	1,865 ft-#
Mu: Design	= 16,120	1,865 ft-#
Actual 1-Way Shear	= 37.69	15.97 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.61 in, #5@ 8.70 in, #6@ 12.36 in, #7@ 16.85 in, #8@ 22.19 in, #9@ 28.
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	2.13	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 7.41 in		#4@ 14.81 in
#5@ 11.48 in		#5@ 22.96 in
#6@ 16.30 in		#6@ 32.59 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	2,531.3	3.75	9,492.2	Soil Over HL (ab. water tbl)	1,796.0	5.83	10,473.7
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		5.83	10,473.7
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	900.0	6.25	5,625.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	170.0	2.13	361.3
				Surcharge Over Toe =			
				Stem Weight(s) =	1,312.5	4.67	6,125.0
				Earth @ Stem Transitions =			
Total	= 3,431.3	O.T.M. =	15,117.2	Footing Weight =	1,233.8	3.29	4,059.0
				Key Weight =			
				Vert. Component =			
				Total =	4,512.3 lbs	R.M.=	21,019.0

Resisting/Overturning Ratio = 1.39
 Vertical Loads used for Soil Pressure = 4,512.3 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.102 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

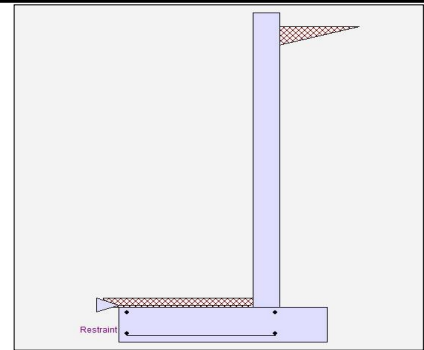
Cantilevered Retaining Wall

Criteria

Retained Height	=	10.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footings Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	2.21 OK
Slab Resists All Sliding !		

Total Bearing Load	=	4,512 lbs
...resultant ecc.	=	8.83 in

Soil Pressure @ Toe	=	1,146 psf OK
Soil Pressure @ Heel	=	226 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,604 psf
ACI Factored @ Heel	=	316 psf
Footing Shear @ Toe	=	23.7 psi OK
Footing Shear @ Heel	=	11.5 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,531.3 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 5
Rebar Spacing	=	10.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.822
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	3,200.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	10,666.7
Moment....Allowable	=	12,971.1

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	32.6
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	8.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	125.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.3012 in ² /ft		
(4/3) * As :	0.4017 in ² /ft	Min Stem T&S Reinf Area 2.520 in ²	
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft	
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.3275 in ² /ft	#4@ 10.00 in	#4@ 20.00 in
Provided Area :	0.372 in ² /ft	#5@ 15.50 in	#5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in	#6@ 44.00 in

Footing Data

Toe Width	=	4.25 ft
Heel Width	=	2.33
Total Footing Width	=	6.58
Footing Thickness	=	15.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,604	316 psf
Mu' : Upward	= 143,776	464 ft-#
Mu' : Downward	= 29,586	1,865 ft-#
Mu: Design	= 9,516	1,401 ft-#
Actual 1-Way Shear	= 23.68	11.54 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 10.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 7.40 in, #5@ 11.48 in, #6@ 16.29 in, #7@ 22.22 in, #8@ 29.25 in, #9@ 37
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	2.13	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 7.41 in		#4@ 14.81 in
#5@ 11.48 in		#5@ 22.96 in
#6@ 16.30 in		#6@ 32.59 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	2,531.3	3.75	9,492.2	Soil Over HL (ab. water tbl)	1,796.0	5.83	10,473.7
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		5.83	10,473.7
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	170.0	2.13	361.3
				Surcharge Over Toe =			
				Stem Weight(s) =	1,312.5	4.67	6,125.0
				Earth @ Stem Transitions =			
				Footing Weight =	1,233.8	3.29	4,059.0
				Key Weight =			
				Vert. Component =			
Total	= 2,531.3	O.T.M. =	9,492.2	Total =	4,512.3 lbs	R.M.=	21,019.0

Resisting/Overturning Ratio = 2.21
 Vertical Loads used for Soil Pressure = 4,512.3 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.051 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

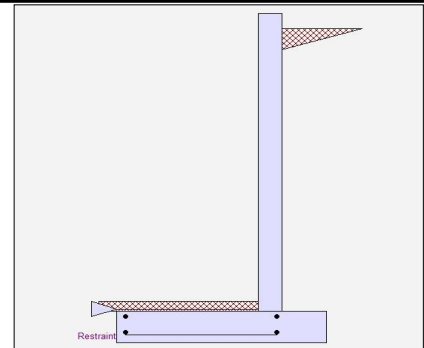
Cantilevered Retaining Wall

Criteria

Retained Height	=	9.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	81.0 #/ft
...Height to Top	=	9.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.33 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	3,339 lbs
...resultant ecc.	=	22.95 in

Soil Pressure @ Toe	=	2,135 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	2,990 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	38.0 psi OK
Footing Shear @ Heel	=	15.3 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,729.0 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.937
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	3,321.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	11,056.5
Moment....Allowable	=	11,799.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	44.7
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.4187 in ² /ft	
(4/3) * As :	0.5582 in ² /ft	Min Stem T&S Reinf Area 1.824 in ²
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.4187 in ² /ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.465 in ² /ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8382 in ² /ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	1.91
Total Footing Width	=	5.91
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,990	0 psf
Mu' : Upward	= 165,915	0 ft-#
Mu' : Downward	= 21,888	1,141 ft-#
Mu: Design	= 12,002	1,141 ft-#
Actual 1-Way Shear	= 37.96	15.29 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 7.05 in, #5@ 10.94 in, #6@ 15.52 in, #7@ 21.17 in, #8@ 27.88 in, #9@ 35
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	1.53	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 9.26 in		#4@ 18.52 in
#5@ 14.35 in		#5@ 28.70 in
#6@ 20.37 in		#6@ 40.74 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	2,000.0	3.33	6,666.7	Soil Over HL (ab. water tbl)	1,342.8	5.29	7,101.2
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		5.29	7,101.2
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	729.0	5.50	4,009.5	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	160.0	2.00	320.0
				Surcharge Over Toe =			
				Stem Weight(s) =	950.0	4.33	4,116.7
				Earth @ Stem Transitions =			
Total	= 2,729.0	O.T.M. =	10,676.2	Footing Weight =	886.5	2.96	2,619.6
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.33		Total =	3,339.3 lbs	R.M.=	14,157.4
Vertical Loads used for Soil Pressure =		3,339.3 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.095 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

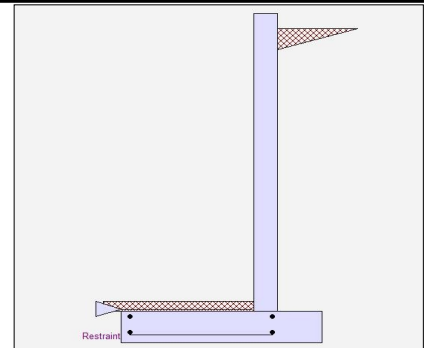
Cantilevered Retaining Wall

Criteria

Retained Height	=	9.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.00 OK
Slab Resists All Sliding !		

Total Bearing Load	=	3,292 lbs
...resultant ecc.	=	9.67 in

Soil Pressure @ Toe	=	1,079 psf OK
Soil Pressure @ Heel	=	85 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,510 psf
ACI Factored @ Heel	=	118 psf
Footing Shear @ Toe	=	26.6 psi OK
Footing Shear @ Heel	=	12.5 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,000.0 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.659
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,592.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	7,776.0
Moment....Allowable	=	11,799.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	34.9
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.2944 in ² /ft		
(4/3) * As :	0.3926 in ² /ft	Min Stem T&S Reinf Area 1.824 in ²	
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of : Two layers of :	
Required Area :	0.2944 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.465 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8382 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	3.75 ft
Heel Width	=	1.91
Total Footing Width	=	5.66
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,510	118 psf
Mu' : Upward	= 101,481	170 ft-#
Mu' : Downward	= 19,238	1,141 ft-#
Mu: Design	= 6,854	971 ft-#
Actual 1-Way Shear	= 26.60	12.48 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 10.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: No key defined

Min footing T&S reinf Area	1.47	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	2,000.0	3.33	6,666.7	Soil Over HL (ab. water tbl)	1,342.8	5.04	6,765.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		5.04	6,765.5
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	150.0	1.88	281.3
				Surcharge Over Toe =			
				Stem Weight(s) =	950.0	4.08	3,879.2
				Earth @ Stem Transitions =			
Total	= 2,000.0	O.T.M. =	6,666.7	Footing Weight =	849.0	2.83	2,402.7
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 2.00		Total =	3,291.8 lbs	R.M.=	13,328.6
Vertical Loads used for Soil Pressure =		3,291.8 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.050 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

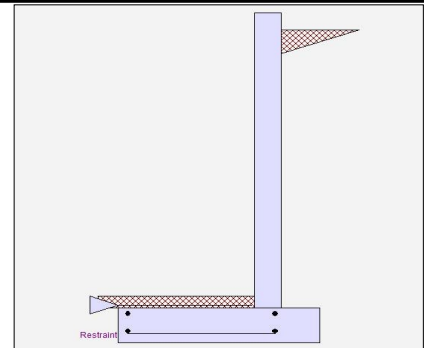
Cantilevered Retaining Wall

Criteria

Retained Height	=	8.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	4.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	150.0 psf/ft
Soil Density, Heel	=	120.00 pcf
Soil Density, Toe	=	120.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	72.0 #/ft
...Height to Top	=	8.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Seismic (E) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.29 Ratio < 1.5!
Slab Resists All Sliding !		

Total Bearing Load	=	2,718 lbs
...resultant ecc.	=	21.21 in

Soil Pressure @ Toe	=	2,231 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	3,123 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	30.6 psi OK
Footing Shear @ Heel	=	11.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,196.0 lbs
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Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.881
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,624.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	7,765.3
Moment....Allowable	=	8,809.0

Shear....Actual

Service Level	psi =	
Strength Level	psi =	35.3
Shear....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.294 in ² /ft		
(4/3) * As :	0.392 in ² /ft	Min Stem T&S Reinf Area 1.632 in ²	
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.294 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.3382 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8382 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	3.50 ft
Heel Width	=	1.66
Total Footing Width	=	5.16
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 3,123	0 psf
Mu' : Upward	= 122,714	0 ft-#
Mu' : Downward	= 16,758	657 ft-#
Mu: Design	= 8,830	657 ft-#
Actual 1-Way Shear	= 30.62	11.03 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 7.50 in, #5@ 11.63 in, #6@ 16.51 in, #7@ 22.51 in, #8@ 29.64 in, #9@ 37
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	1.34	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,620.0	3.00	4,860.0	Soil Over HL (ab. water tbl)	953.6	4.66	4,447.0
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.66	4,447.0
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	576.0	5.00	2,880.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	140.0	1.75	245.0
				Surcharge Over Toe =			
				Stem Weight(s) =	850.0	3.83	3,258.3
				Earth @ Stem Transitions =			
Total	= 2,196.0	O.T.M. =	7,740.0	Footing Weight =	774.0	2.58	1,996.9
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.29		Total =	2,717.6 lbs	R.M.=	9,947.2
Vertical Loads used for Soil Pressure =		2,717.6 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.102 in

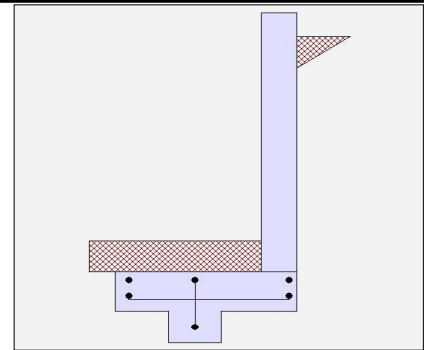
The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall**Criteria**

Retained Height	=	5.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footings Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.16 OK
Sliding	=	1.57 OK
Total Bearing Load	=	1,281 lbs
...resultant ecc.	=	6.12 in
Soil Pressure @ Toe	=	710 psf OK
Soil Pressure @ Heel	=	39 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	994 psf
ACI Factored @ Heel	=	55 psf
Footing Shear @ Toe	=	11.1 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi
Sliding Calcs		
Lateral Sliding Force	=	680.6 lbs
less 100% Passive Force	= -	554.2 lbs
less 100% Friction Force	= -	512.4 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge
Design Data		
fb/FB + fa/Fa	=	0.240
Total Force @ Section		
Service Level	lbs =	
Strength Level	lbs =	850.0
Moment....Actual		
Service Level	ft-# =	
Strength Level	ft-# =	1,416.7
Moment....Allowable	=	5,883.6
Shear.....Actual		
Service Level	psi =	
Strength Level	psi =	11.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0531 in ² /ft		
(4/3) * As :	0.0708 in ² /ft	Min Stem T&S Reinf Area 1.056 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2182 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	2.75 ft
Heel Width	=	0.67
Total Footing Width	=	3.42
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	8.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 994	55 psf
Mu' : Upward	= 33,672	0 ft-#
Mu' : Downward	= 12,599	0 ft-#
Mu: Design	= 1,756	0 ft-#
Actual 1-Way Shear	= 11.07	0.03 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.26 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	0.74	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	680.6	1.94	1,323.3	Soil Over HL (ab. water tbl)	1.8	3.42	6.3
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.42	6.3
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	201.7	1.38	277.3
				Surcharge Over Toe =			
				Stem Weight(s) =	550.0	3.08	1,695.8
				Earth @ Stem Transitions =			
				Footing Weight =	427.5	1.71	731.0
				Key Weight =	100.0	1.50	150.0
				Vert. Component =			
Total	= 680.6	O.T.M. =	1,323.3	Total =	1,281.0 lbs	R.M.=	2,860.4
Resisting/Overturning Ratio		=	2.16				
Vertical Loads used for Soil Pressure =		1,281.0 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.032 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

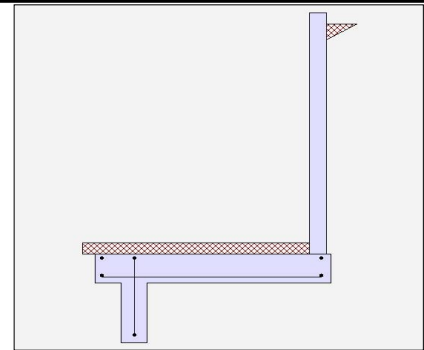
Cantilevered Retaining Wall

Criteria

Retained Height	=	10.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.20 OK
Sliding	=	1.65 OK
Total Bearing Load	=	3,778 lbs
...resultant ecc.	=	18.42 in
Soil Pressure @ Toe	=	838 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,173 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	18.1 psi OK
Footing Shear @ Heel	=	1.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,531.3 lbs
less 100% Passive Force	= -	2,666.7 lbs
less 100% Friction Force	= -	1,511.1 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 6
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.787
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	3,400.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	11,333.3
Moment....Allowable	=	14,393.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	50.4
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	5.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

	Vertical Reinforcing	Horizontal Reinforcing
Bottom Stem		
As (based on applied moment) :	0.4746 in ² /ft	
(4/3) * As :	0.6328 in ² /ft	Min Stem T&S Reinf Area 2.016 in ²
200bd/fy : 200(12)(5.625)/60000 :	0.225 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.4746 in ² /ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.66 in ² /ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.762 in ² /ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	8.25 ft
Heel Width	=	0.83
Total Footing Width	=	9.08
Footing Thickness	=	15.00 in
Key Width	=	12.00 in
Key Depth	=	31.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,173	0 psf
Mu' : Upward	= 332,890	0 ft-#
Mu' : Downward	= 138,643	25 ft-#
Mu: Design	= 16,187	25 ft-#
Actual 1-Way Shear	= 18.11	1.92 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 5 @ 12.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.59 in, #5@ 8.67 in, #6@ 12.30 in, #7@ 16.78 in, #8@ 22.10 in, #9@ 27.
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 10 in, #5@ 15.5 in, #6@ 18 in, #7@ 18 in, #8@ 18 in

Min footing T&S reinf Area	2.94	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 7.41 in		#4@ 14.81 in
#5@ 11.48 in		#5@ 22.96 in
#6@ 16.30 in		#6@ 32.59 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	2,531.3	3.75	9,492.2	Soil Over HL (ab. water tbl)	183.3	9.00	1,650.0
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		9.00	1,650.0
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	453.8	4.13	1,871.7
				Surcharge Over Toe =			
				Stem Weight(s) =	1,050.0	8.58	9,012.5
				Earth @ Stem Transitions =			
Total	= 2,531.3	O.T.M. =	9,492.2	Footing Weight =	1,703.1	4.54	7,735.0
				Key Weight =	387.5	1.50	581.3
				Vert. Component =			
				Total =	3,777.7 lbs	R.M.=	20,850.5

Resisting/Overturning Ratio = 2.20
 Vertical Loads used for Soil Pressure = 3,777.7 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.027 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

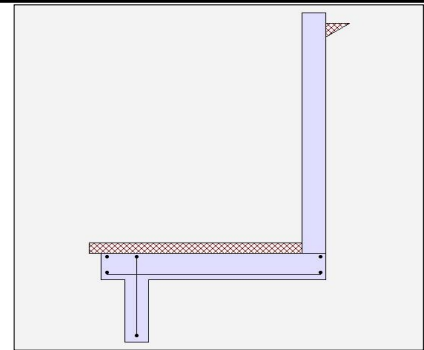
Cantilevered Retaining Wall

Criteria

Retained Height	=	11.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footings Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.17 OK
Sliding	=	1.67 OK
Total Bearing Load	=	4,424 lbs
...resultant ecc.	=	17.96 in
Soil Pressure @ Toe	=	906 psf OK
Soil Pressure @ Heel	=	26 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,268 psf
ACI Factored @ Heel	=	36 psf
Footing Shear @ Toe	=	23.1 psi OK
Footing Shear @ Heel	=	1.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	3,001.3 lbs
less 100% Passive Force	= -	3,234.4 lbs
less 100% Friction Force	= -	1,769.5 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	12.00
Rebar Size	=	# 6
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.574
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	4,114.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	15,084.7
Moment....Allowable	=	26,273.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	35.6
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	9.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	150.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.3602 in ² /ft	
(4/3) * As :	0.4803 in ² /ft	Min Stem T&S Reinf Area 3.312 in ²
200bd/fy : 200(12)(9.625)/60000 :	0.385 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.288 in ² /ft
0.0018bh : 0.0018(12)(12) :	0.2592 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.385 in ² /ft	#4@ 8.33 in #4@ 16.67 in
Provided Area :	0.66 in ² /ft	#5@ 12.92 in #5@ 25.83 in
Maximum Area :	1.3039 in ² /ft	#6@ 18.33 in #6@ 36.67 in

Footing Data

Toe Width	=	8.50 ft
Heel Width	=	1.00
Total Footing Width	=	9.50
Footing Thickness	=	15.00 in
Key Width	=	12.00 in
Key Depth	=	36.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,268	36 psf
Mu' : Upward	= 390,388	0 ft-#
Mu' : Downward	= 147,173	0 ft-#
Mu: Design	= 20,268	0 ft-#
Actual 1-Way Shear	= 23.11	1.92 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 5 @ 10.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.21 in, #5@ 8.08 in, #6@ 11.47 in, #7@ 15.65 in, #8@ 20.60 in, #9@ 26.
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 7.29 in, #5@ 11.31 in, #6@ 16.05 in, #7@ 18 in, #8@

Min footing T&S reinf Area	3.08	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 7.41 in		#4@ 14.81 in
#5@ 11.48 in		#5@ 22.96 in
#6@ 16.30 in		#6@ 32.59 in

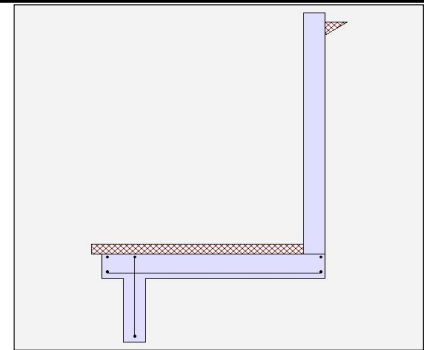
Cantilevered Retaining Wall

Criteria

Retained Height	=	12.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.01 OK
Sliding	=	1.61 OK
Total Bearing Load	=	4,806 lbs
...resultant ecc.	=	22.23 in
Soil Pressure @ Toe	=	979 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,371 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	26.2 psi OK
Footing Shear @ Heel	=	1.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	3,511.3 lbs
less 100% Passive Force	= -	3,726.0 lbs
less 100% Friction Force	= -	1,922.3 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	12.00
Rebar Size	=	# 6
Rebar Spacing	=	7.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.660
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	4,896.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	19,584.0
Moment....Allowable	=	29,648.9

Shear....Actual

Service Level	psi =	
Strength Level	psi =	42.4
Shear....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	9.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	150.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

	Vertical Reinforcing	Horizontal Reinforcing
Bottom Stem		
As (based on applied moment) :	0.4676 in ² /ft	
(4/3) * As :	0.6235 in ² /ft	Min Stem T&S Reinf Area 3.600 in ²
200bd/fy : 200(12)(9.625)/60000 :	0.385 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.288 in ² /ft
0.0018bh : 0.0018(12)(12) :	0.2592 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.4676 in ² /ft	#4@ 8.33 in #4@ 16.67 in
Provided Area :	0.7543 in ² /ft	#5@ 12.92 in #5@ 25.83 in
Maximum Area :	1.3039 in ² /ft	#6@ 18.33 in #6@ 36.67 in

Footing Data

Toe Width	=	9.25 ft
Heel Width	=	1.00
Total Footing Width	=	10.25
Footing Thickness	=	15.00 in
Key Width	=	12.00 in
Key Depth	=	40.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,371	0 psf
Mu' : Upward	= 482,651	0 ft-#
Mu' : Downward	= 174,291	0 ft-#
Mu: Design	= 25,697	0 ft-#
Actual 1-Way Shear	= 26.19	1.92 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 7.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 5 @ 8.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 4.69 in, #5@ 7.28 in, #6@ 10.33 in, #7@ 14.09 in, #8@ 18.56 in, #9@ 23.
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 5.71 in, #5@ 8.85 in, #6@ 12.56 in, #7@ 17.13 in, #

Min footing T&S reinf Area	3.32	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 7.41 in		#4@ 14.81 in
#5@ 11.48 in		#5@ 22.96 in
#6@ 16.30 in		#6@ 32.59 in

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	3,511.3	4.42	15,508.0	Soil Over HL (ab. water tbl)			
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)			
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	508.8	4.63	2,353.0
				Surcharge Over Toe =			
				Stem Weight(s) =	1,875.0	9.75	18,281.3
				Earth @ Stem Transitions =			
				Footing Weight =	1,921.9	5.13	9,849.6
				Key Weight =	500.0	1.50	750.0
				Vert. Component =			
Total	= 3,511.3	O.T.M. =	15,508.0	Total =	4,805.6 lbs	R.M.=	31,233.8
Resisting/Overturning Ratio		=	2.01				
Vertical Loads used for Soil Pressure =		4,805.6 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.033 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

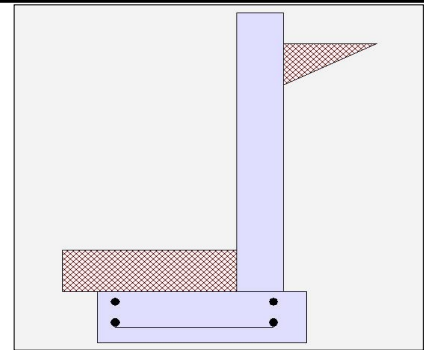
Cantilevered Retaining Wall

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	250.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	36.0 #/ft
...Height to Top	=	4.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.87 OK
Sliding	=	1.35 Ratio < 1.5!
Total Bearing Load	=	1,118 lbs
...resultant ecc.	=	7.12 in
Soil Pressure @ Toe	=	822 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,151 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	10.2 psi OK
Footing Shear @ Heel	=	2.7 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	611.2 lbs
less 100% Passive Force	= -	375.0 lbs
less 100% Friction Force	= -	447.3 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	94.5 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.203
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	731.2

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	1,099.7
Moment....Allowable	=	5,412.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	9.7
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in ² =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0412 in ² /ft		
(4/3) * As :	0.0549 in ² /ft	Min Stem T&S Reinf Area 0.864 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	2.00 ft
Heel Width	=	1.00
Total Footing Width	=	3.00
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	6.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,151	0 psf
Mu' : Upward	= 20,857	0 ft-#
Mu' : Downward	= 6,664	44 ft-#
Mu: Design	= 1,183	44 ft-#
Actual 1-Way Shear	= 10.15	2.74 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	0.65	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	467.2	1.61	752.7	Soil Over HL (ab. water tbl)	146.7	2.83	415.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.83	415.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	144.0	2.83	408.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	146.7	1.00	146.7
				Surcharge Over Toe =			
				Stem Weight(s) =	450.0	2.33	1,050.0
				Earth @ Stem Transitions =			
Total	= 611.2	O.T.M. =	1,160.7	Footing Weight =	375.0	1.50	562.5
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio		= 1.87		Total =	1,118.3 lbs	R.M.=	2,174.7
Vertical Loads used for Soil Pressure =		1,118.3 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.034 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Cantilevered Retaining Wall

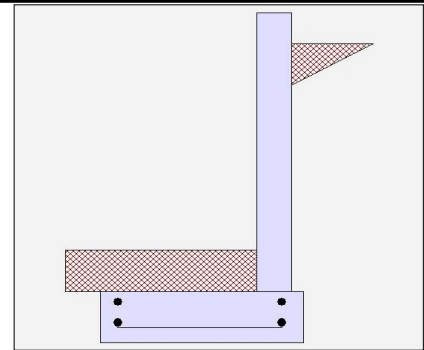
Code: IBC 2015,ACI 318-14,ACI 530-13

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	36.0 #/ft
...Height to Top	=	4.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.52 OK
Sliding	=	1.35 Ratio < 1.5!
Total Bearing Load	=	940 lbs
...resultant ecc.	=	9.74 in
Soil Pressure @ Toe	=	970 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,357 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	10.1 psi OK
Footing Shear @ Heel	=	1.4 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	611.2 lbs
less 100% Passive Force	= -	450.0 lbs
less 100% Friction Force	= -	376.2 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	90.7 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	18.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.447
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	731.2

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	1,099.7
Moment....Allowable	=	2,455.6

Shear....Actual

Service Level	psi =	
Strength Level	psi =	14.3
Shear....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0622 in ² /ft		
(4/3) * As :	0.0829 in ² /ft	Min Stem T&S Reinf Area 0.648 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1296 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.1333 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	2.25 ft
Heel Width	=	0.67
Total Footing Width	=	2.92
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	6.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,357	0 psf
Mu' : Upward	= 25,332	0 ft-#
Mu' : Downward	= 8,434	11 ft-#
Mu: Design	= 1,408	11 ft-#
Actual 1-Way Shear	= 10.08	1.37 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: No key defined

Min footing T&S reinf Area	0.63	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	467.2	1.61	752.7	Soil Over HL (ab. water tbl)	73.3	2.83	207.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.83	207.8
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	144.0	2.83	408.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	165.0	1.13	185.6
=				Surcharge Over Toe =			
Total =	611.2	O.T.M. =	1,160.7	Stem Weight(s) =	337.5	2.50	843.8
				Earth @ Stem Transitions =			
				Footing Weight =	364.6	1.46	531.7
				Key Weight =			
				Vert. Component =			
				Total =	940.4 lbs	R.M.=	1,768.8

Resisting/Overturning Ratio = 1.52
 Vertical Loads used for Soil Pressure = 940.4 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.042 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

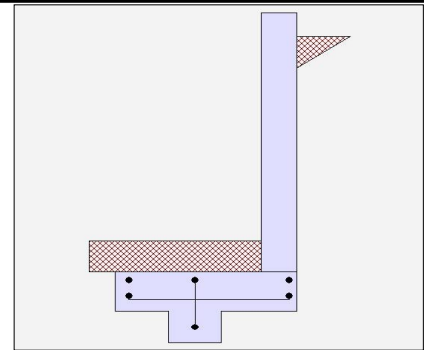
Cantilevered Retaining Wall

Criteria

Retained Height	=	5.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	45.0 #/ft
...Height to Top	=	5.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.38 Ratio < 1.5!
Sliding	=	1.18 Ratio < 1.5!
Total Bearing Load	=	1,281 lbs
...resultant ecc.	=	13.15 in
Soil Pressure @ Toe	=	1,390 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,946 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	14.5 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	905.6 lbs
less 100% Passive Force	= -	554.2 lbs
less 100% Friction Force	= -	512.4 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	291.8 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	11.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.365
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,142.5

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,147.9
Moment....Allowable	=	5,883.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	15.2
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0805 in ² /ft		
(4/3) * As :	0.1073 in ² /ft	Min Stem T&S Reinf Area 1.056 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2182 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	2.75 ft
Heel Width	=	0.67
Total Footing Width	=	3.42
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	8.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,946	0 psf
Mu' : Upward	= 45,959	0 ft-#
Mu' : Downward	= 12,599	0 ft-#
Mu: Design	= 2,780	0 ft-#
Actual 1-Way Shear	= 14.51	0.03 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.26 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	0.74	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	680.6	1.94	1,323.3	Soil Over HL (ab. water tbl)	1.8	3.42	6.3
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.42	6.3
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	225.0	3.33	750.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	201.7	1.38	277.3
				Surcharge Over Toe =			
				Stem Weight(s) =	550.0	3.08	1,695.8
				Earth @ Stem Transitions =			
Total	= 905.6	O.T.M. =	2,073.3	Footing Weight =	427.5	1.71	731.0
				Key Weight =	100.0	1.50	150.0
				Vert. Component =			
Resisting/Overturning Ratio		= 1.38		Total =	1,281.0 lbs	R.M.=	2,860.4
Vertical Loads used for Soil Pressure =		1,281.0 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt**Horizontal Deflection at Top of Wall due to settlement of soil**

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
Horizontal Defl @ Top of Wall (approximate only) 0.062 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

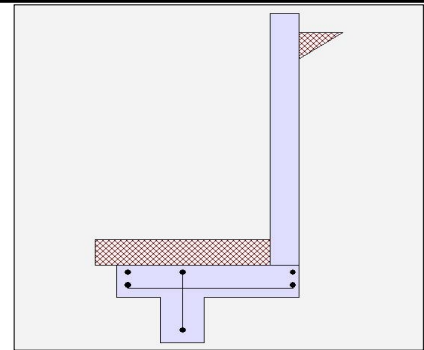
Cantilevered Retaining Wall

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	54.0 #/ft
...Height to Top	=	6.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.28 Ratio < 1.5!
Sliding	=	1.24 Ratio < 1.5!
Total Bearing Load	=	1,605 lbs
...resultant ecc.	=	18.07 in
Soil Pressure @ Toe	=	1,847 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,585 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	17.4 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,257.9 lbs
less 100% Passive Force	= -	916.7 lbs
less 100% Friction Force	= -	642.0 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	328.1 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	10.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.575
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,645.2

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	3,711.6
Moment....Allowable	=	6,444.1

Shear....Actual

Service Level	psi =	
Strength Level	psi =	21.9
Shear....Allowable	psi =	75.0
Anet (Masonry)	in ² =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1391 in ² /ft		
(4/3) * As :	0.1854 in ² /ft	Min Stem T&S Reinf Area 1.248 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1854 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.24 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	3.50 ft
Heel Width	=	0.67
Total Footing Width	=	4.17
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	14.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,585	0 psf
Mu' : Upward	= 78,755	0 ft-#
Mu' : Downward	= 20,409	0 ft-#
Mu: Design	= 4,862	0 ft-#
Actual 1-Way Shear	= 17.44	0.04 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 10.29 in, #5@ 15.96 in, #6@ 22.65 in, #7@ 30.89 in, #8@ 40.67 in, #9@ 5
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	0.90	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	933.9	2.28	2,127.2	Soil Over HL (ab. water tbl)	2.2	4.17	9.2
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.17	9.2
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	324.0	3.83	1,242.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	256.7	1.75	449.2
				Surcharge Over Toe =			
				Stem Weight(s) =	650.0	3.83	2,491.7
				Earth @ Stem Transitions =			
Total	= 1,257.9	O.T.M. =	3,369.2	Footing Weight =	521.3	2.09	1,086.8
				Key Weight =	175.0	1.50	262.5
				Vert. Component =			
				Total =	1,605.1 lbs	R.M.=	4,299.3

Resisting/Overturning Ratio = 1.28
 Vertical Loads used for Soil Pressure = 1,605.1 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.080 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

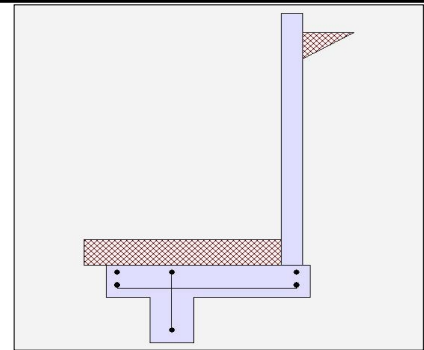
Cantilevered Retaining Wall

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	54.0 #/ft
...Height to Top	=	6.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.42 Ratio < 1.5!
Sliding	=	1.25 Ratio < 1.5!
Total Bearing Load	=	1,649 lbs
...resultant ecc.	=	17.69 in
Soil Pressure @ Toe	=	1,279 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,791 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	16.3 psi OK
Footing Shear @ Heel	=	1.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,257.9 lbs
less 100% Passive Force	= -	916.7 lbs
less 100% Friction Force	= -	659.7 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	310.5 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	9.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.785
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,645.2

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	3,711.6
Moment....Allowable	=	4,722.4

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	32.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.2098 in ² /ft		
(4/3) * As :	0.2797 in ² /ft	Min Stem T&S Reinf Area 0.936 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.2098 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2667 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	0.67
Total Footing Width	=	4.67
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	14.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,791	0 psf
Mu' : Upward	= 87,016	0 ft-#
Mu' : Downward	= 26,656	15 ft-#
Mu: Design	= 5,030	15 ft-#
Actual 1-Way Shear	= 16.31	1.91 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.26 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.95 in, #5@ 15.42 in, #6@ 21.89 in, #7@ 29.86 in, #8@ 39.31 in, #9@ 49
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	1.01	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	933.9	2.28	2,127.2	Soil Over HL (ab. water tbl)	110.0	4.58	504.2
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.58	504.2
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	324.0	3.83	1,242.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	293.3	2.00	586.7
=				Surcharge Over Toe =			
Total =	1,257.9	O.T.M. =	3,369.2	Stem Weight(s) =	487.5	4.25	2,071.9
				Earth @ Stem Transitions =			
				Footing Weight =	583.3	2.33	1,361.1
				Key Weight =	175.0	1.50	262.5
				Vert. Component =			
				Total =	1,649.2 lbs	R.M.=	4,786.3
Resisting/Overturning Ratio =			1.42	* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			
Vertical Loads used for Soil Pressure =		1,649.2 lbs					

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.050 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

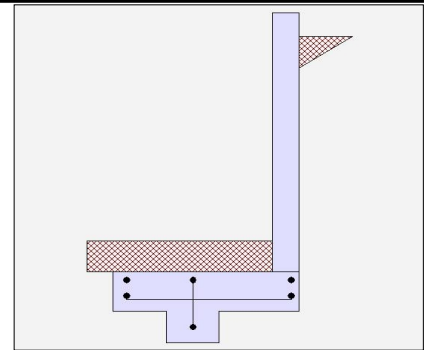
Cantilevered Retaining Wall

Criteria

Retained Height	=	5.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	45.0 #/ft
...Height to Top	=	5.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.25 Ratio < 1.5!
Sliding	=	1.13 Ratio < 1.5!
Total Bearing Load	=	1,170 lbs
...resultant ecc.	=	15.74 in
Soil Pressure @ Toe	=	1,779 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,491 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	11.6 psi OK
Footing Shear @ Heel	=	1.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	905.6 lbs
less 100% Passive Force	= -	554.2 lbs
less 100% Friction Force	= -	468.0 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	336.2 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.594
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,142.5

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,147.9
Moment....Allowable	=	3,612.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	22.4
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1214 in ² /ft		
(4/3) * As :	0.1619 in ² /ft	Min Stem T&S Reinf Area 0.792 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1619 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	3.00 ft
Heel Width	=	0.50
Total Footing Width	=	3.50
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	8.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,491	0 psf
Mu' : Upward	= 50,350	0 ft-#
Mu' : Downward	= 14,994	0 ft-#
Mu: Design	= 2,946	0 ft-#
Actual 1-Way Shear	= 11.56	1.91 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.26 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	0.76	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

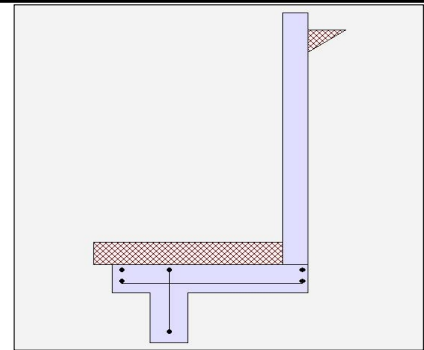
Cantilevered Retaining Wall

Criteria

Retained Height	=	7.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	63.0 #/ft
...Height to Top	=	7.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.25 Ratio < 1.5!
Sliding	=	1.19 Ratio < 1.5!
Total Bearing Load	=	1,954 lbs
...resultant ecc.	=	23.20 in
Soil Pressure @ Toe	=	1,999 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,798 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	19.9 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,668.2 lbs
less 100% Passive Force	= -	1,200.0 lbs
less 100% Friction Force	= -	781.5 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	520.8 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.740
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,239.3

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	5,893.9
Moment....Allowable	=	7,959.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	29.9
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in ² =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.2208 in ² /ft		
(4/3) * As :	0.2944 in ² /ft	Min Stem T&S Reinf Area 1.440 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.25 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.3 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	4.50 ft
Heel Width	=	0.67
Total Footing Width	=	5.17
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	18.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,798	0 psf
Mu' : Upward	= 126,315	0 ft-#
Mu' : Downward	= 33,737	0 ft-#
Mu: Design	= 7,715	0 ft-#
Actual 1-Way Shear	= 19.86	0.04 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 8.65 in, #5@ 13.41 in, #6@ 19.03 in, #7@ 25.95 in, #8@ 34.17 in, #9@ 43
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	1.12	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	1,227.2	2.61	3,204.4	Soil Over HL (ab. water tbl)	2.6	13.3
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)	5.17	13.3
Hydrostatic Force				Watre Table		
Buoyant Force =				Sloped Soil Over Heel =		
Surcharge over Heel =				Surcharge Over Heel =		
Surcharge Over Toe =				Adjacent Footing Load =		
Adjacent Footing Load =				Axial Dead Load on Stem =		
Added Lateral Load =	441.0	4.33	1,911.0	* Axial Live Load on Stem =		
Load @ Stem Above Soil =				Soil Over Toe =	330.0	742.5
				Surcharge Over Toe =		
				Stem Weight(s) =	750.0	3,625.0
				Earth @ Stem Transitions =		
Total	= 1,668.2	O.T.M. =	5,115.4	Footing Weight =	646.3	1,670.6
				Key Weight =	225.0	337.5
				Vert. Component =		
Resisting/Overturning Ratio		= 1.25		Total =	1,953.8 lbs	R.M.= 6,388.8
Vertical Loads used for Soil Pressure =		1,953.8 lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.081 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

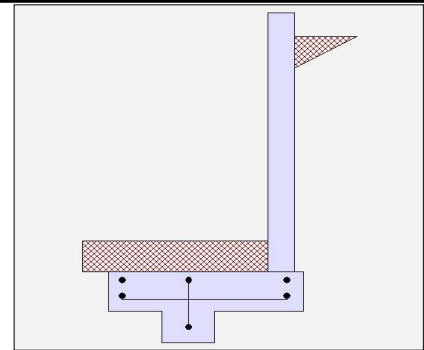
Cantilevered Retaining Wall

Criteria

Retained Height	=	5.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.26 OK
Sliding	=	1.57 OK
Total Bearing Load	=	1,283 lbs
...resultant ecc.	=	6.41 in
Soil Pressure @ Toe	=	656 psf OK
Soil Pressure @ Heel	=	44 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	918 psf
ACI Factored @ Heel	=	62 psf
Footing Shear @ Toe	=	10.4 psi OK
Footing Shear @ Heel	=	1.5 psi OK
Allowable	=	75.0 psi
Sliding Calcs		
Lateral Sliding Force	=	680.6 lbs
less 100% Passive Force	= -	554.2 lbs
less 100% Friction Force	= -	513.0 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	16.00
Rebar Placed at	=	Edge
Design Data		
fb/FB + fa/Fa	=	0.515
Total Force @ Section		
Service Level	lbs =	
Strength Level	lbs =	850.0
Moment....Actual		
Service Level	ft-# =	
Strength Level	ft-# =	1,416.7
Moment....Allowable	=	2,749.3
Shear.....Actual		
Service Level	psi =	
Strength Level	psi =	16.7
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0801 in ² /ft		
(4/3) * As :	0.1068 in ² /ft	Min Stem T&S Reinf Area 0.792 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1296 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.15 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	3.00 ft
Heel Width	=	0.67
Total Footing Width	=	3.67
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	8.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

		Toe	Heel
Factored Pressure	=	918	62 psf
Mu' : Upward	=	36,951	1 ft-#
Mu' : Downward	=	14,994	13 ft-#
Mu: Design	=	1,830	12 ft-#
Actual 1-Way Shear	=	10.44	1.50 psi
Allow 1-Way Shear	=	75.00	40.00 psi
Toe Reinforcing	=	# 4 @ 11.11 in	
Heel Reinforcing	=	None Spec'd	
Key Reinforcing	=	# 4 @ 9.26 in	
Footing Torsion, Tu	=		0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=		0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	0.79	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	680.6	1.94	1,323.3	Soil Over HL (ab. water tbl)	91.7	3.58	328.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.58	328.5
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	220.0	1.50	330.0
				Surcharge Over Toe =			
				Stem Weight(s) =	412.5	3.25	1,340.6
				Earth @ Stem Transitions =			
Total	= 680.6	O.T.M. =	1,323.3	Footing Weight =	458.3	1.83	840.3
				Key Weight =	100.0	1.50	150.0
				Vert. Component =			
				Total =	1,282.5 lbs	R.M.=	2,989.4

Resisting/Overturning Ratio = 2.26
 Vertical Loads used for Soil Pressure = 1,282.5 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.027 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

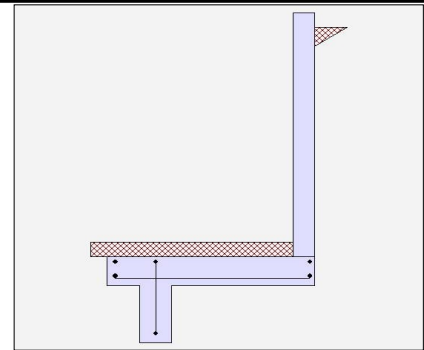
Cantilevered Retaining Wall

Criteria

Retained Height	=	8.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footings Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	72.0 #/ft
...Height to Top	=	8.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.25 Ratio < 1.5!
Sliding	=	1.21 Ratio < 1.5!
Total Bearing Load	=	2,432 lbs
...resultant ecc.	=	29.15 in
Soil Pressure @ Toe	=	2,076 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,906 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	18.4 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,196.0 lbs
less 100% Passive Force	= -	1,687.5 lbs
less 100% Friction Force	= -	972.9 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	633.6 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015, ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	10.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.914
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,924.8

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	8,797.9
Moment....Allowable	=	9,623.1

Shear....Actual

Service Level	psi =	
Strength Level	psi =	39.4
Shear....Allowable	psi =	75.0
Anet (Masonry)	in ² =	
Rebar Depth 'd'	in =	6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.3331 in ² /ft		
(4/3) * As :	0.4442 in ² /ft	Min Stem T&S Reinf Area 1.632 in ²	
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.3331 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.372 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8382 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	5.75 ft
Heel Width	=	0.67
Total Footing Width	=	6.42
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	24.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,906	0 psf
Mu' : Upward	= 203,027	0 ft-#
Mu' : Downward	= 56,934	0 ft-#
Mu: Design	= 12,174	0 ft-#
Actual 1-Way Shear	= 18.38	0.04 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 10.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 7.05 in, #5@ 10.94 in, #6@ 15.52 in, #7@ 21.17 in, #8@ 27.88 in, #9@ 35
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	1.66	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	1,620.0	3.00	4,860.0	Soil Over HL (ab. water tbl)	2.9	18.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)	6.42	18.8
Hydrostatic Force				Watre Table		
Buoyant Force =				Sloped Soil Over Heel =		
Surcharge over Heel =				Surcharge Over Heel =		
Surcharge Over Toe =				Adjacent Footing Load =		
Adjacent Footing Load =				Axial Dead Load on Stem =		
Added Lateral Load =	576.0	5.00	2,880.0	* Axial Live Load on Stem =		
Load @ Stem Above Soil =				Soil Over Toe =	316.3	909.2
				Surcharge Over Toe =		
				Stem Weight(s) =	850.0	5,170.8
				Earth @ Stem Transitions =		
Total	= 2,196.0	O.T.M. =	7,740.0	Footing Weight =	963.0	3,091.2
				Key Weight =	300.0	450.0
				Vert. Component =		
				Total =	2,432.2 lbs	R.M.= 9,640.1

Resisting/Overturning Ratio = 1.25
 Vertical Loads used for Soil Pressure = 2,432.2 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.076 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

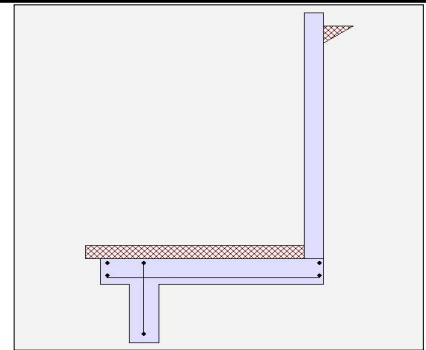
Cantilevered Retaining Wall

Criteria

Retained Height	=	9.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	81.0 #/ft
...Height to Top	=	9.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.24 Ratio < 1.5!
Sliding	=	1.13 Ratio < 1.5!
Total Bearing Load	=	2,826 lbs
...resultant ecc.	=	35.06 in
Soil Pressure @ Toe	=	2,063 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,888 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	20.1 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,729.0 lbs
less 100% Passive Force	= -	1,959.4 lbs
less 100% Friction Force	= -	1,130.5 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	1,003.6 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	6.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.822
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	3,701.7

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	12,526.7
Moment....Allowable	=	15,222.0

Shear....Actual

Service Level	psi =	
Strength Level	psi =	49.9
Shear....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.4743 in ² /ft		
(4/3) * As :	0.6324 in ² /ft	Min Stem T&S Reinf Area 1.824 in ²	
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.4743 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.62 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8382 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	7.00 ft
Heel Width	=	0.67
Total Footing Width	=	7.67
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	27.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,888	0 psf
Mu' : Upward	= 289,000	0 ft-#
Mu' : Downward	= 84,378	0 ft-#
Mu: Design	= 17,052	0 ft-#
Actual 1-Way Shear	= 20.07	0.04 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.18 in, #5@ 8.03 in, #6@ 11.39 in, #7@ 15.54 in, #8@ 20.46 in, #9@ 25.
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	1.99	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	2,000.0	3.33	6,666.7	Soil Over HL (ab. water tbl)	3.3	7.67	25.3
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		7.67	25.3
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	729.0	5.50	4,009.5	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	385.0	3.50	1,347.5
				Surcharge Over Toe =			
				Stem Weight(s) =	950.0	7.33	6,966.7
				Earth @ Stem Transitions =			
Total	= 2,729.0	O.T.M. =	10,676.2	Footing Weight =	1,150.5	3.84	4,412.2
				Key Weight =	337.5	1.50	506.3
				Vert. Component =			
				Total =	2,826.3 lbs	R.M.=	13,257.9

Resisting/Overturning Ratio = 1.24
 Vertical Loads used for Soil Pressure = 2,826.3 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.071 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

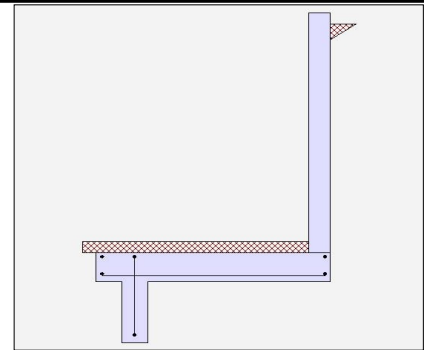
Cantilevered Retaining Wall

Criteria

Retained Height	=	10.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footings Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	90.0 #/ft
...Height to Top	=	10.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.43 Ratio < 1.5!
Sliding	=	1.26 Ratio < 1.5!
Total Bearing Load	=	3,869 lbs
...resultant ecc.	=	34.45 in
Soil Pressure @ Toe	=	1,544 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,162 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	20.4 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	3,431.3 lbs
less 100% Passive Force	= -	2,776.0 lbs
less 100% Friction Force	= -	1,547.8 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	823.1 lbs NG

Vertical component of active lateral soil pressure IS
NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 6
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.845
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	4,570.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	17,183.3
Moment....Allowable	=	20,333.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	49.9
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in ² =	
Rebar Depth 'd'	in =	7.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	125.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.5226 in ² /ft		
(4/3) * As :	0.6968 in ² /ft	Min Stem T&S Reinf Area 2.520 in ²	
200bd/fy : 200(12)(7.625)/60000 :	0.305 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft	
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.5226 in ² /ft	#4@ 10.00 in	#4@ 20.00 in
Provided Area :	0.66 in ² /ft	#5@ 15.50 in	#5@ 31.00 in
Maximum Area :	1.0329 in ² /ft	#6@ 22.00 in	#6@ 44.00 in

Footing Data

Toe Width	=	8.25 ft
Heel Width	=	0.83
Total Footing Width	=	9.08
Footing Thickness	=	15.00 in
Key Width	=	12.00 in
Key Depth	=	32.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,162	0 psf
Mu' : Upward	= 427,691	0 ft-#
Mu' : Downward	= 138,643	0 ft-#
Mu: Design	= 24,087	0 ft-#
Actual 1-Way Shear	= 20.44	0.03 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 5 @ 12.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.01 in, #5@ 7.77 in, #6@ 11.02 in, #7@ 15.03 in, #8@ 19.80 in, #9@ 25.
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.64 in, #5@ 14.94 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	2.94	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 7.41 in	#4@ 14.81 in	
#5@ 11.48 in	#5@ 22.96 in	
#6@ 16.30 in	#6@ 32.59 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	2,531.3	3.75	9,492.2	Soil Over HL (ab. water tbl)	0.0	9.08	0.0
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		9.08	0.0
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =	900.0	6.25	5,625.0	* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	453.8	4.13	1,871.7
				Surcharge Over Toe =			
				Stem Weight(s) =	1,312.5	8.67	11,375.0
				Earth @ Stem Transitions =			
Total	= 3,431.3	O.T.M. =	15,117.2	Footing Weight =	1,703.1	4.54	7,735.0
				Key Weight =	400.0	1.50	600.0
				Vert. Component =			
				Total =	3,869.4 lbs	R.M.=	21,581.7
Resisting/Overturning Ratio		= 1.43					
Vertical Loads used for Soil Pressure =		3,869.4 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.050 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

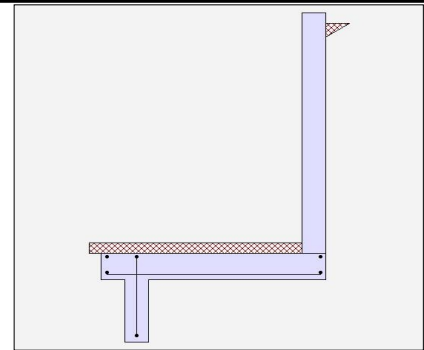
Cantilevered Retaining Wall

Criteria

Retained Height	=	11.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footings Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	99.0 #/ft
...Height to Top	=	11.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.36 Ratio < 1.5!
Sliding	=	1.22 Ratio < 1.5!
Total Bearing Load	=	4,424 lbs
...resultant ecc.	=	37.90 in
Soil Pressure @ Toe	=	1,853 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,594 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	25.4 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	4,090.3 lbs
less 100% Passive Force	= -	3,234.4 lbs
less 100% Friction Force	= -	1,769.5 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	1,131.5 lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	12.00
Rebar Size	=	# 6
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.870
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	5,529.7

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	22,871.0
Moment....Allowable	=	26,273.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	47.9
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in ² =	
Rebar Depth 'd'	in =	9.63

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	150.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.5461 in ² /ft		
(4/3) * As :	0.7282 in ² /ft	Min Stem T&S Reinf Area 3.312 in ²	
200bd/fy : 200(12)(9.625)/60000 :	0.385 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.288 in ² /ft	
0.0018bh : 0.0018(12)(12) :	0.2592 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.5461 in ² /ft	#4@ 8.33 in	#4@ 16.67 in
Provided Area :	0.66 in ² /ft	#5@ 12.92 in	#5@ 25.83 in
Maximum Area :	1.3039 in ² /ft	#6@ 18.33 in	#6@ 36.67 in

Footing Data

Toe Width	=	8.50 ft
Heel Width	=	1.00
Total Footing Width	=	9.50
Footing Thickness	=	15.00 in
Key Width	=	12.00 in
Key Depth	=	36.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 2,594	0 psf
Mu' : Upward	= 513,407	0 ft-#
Mu' : Downward	= 147,173	0 ft-#
Mu: Design	= 30,519	0 ft-#
Actual 1-Way Shear	= 25.42	0.03 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 6 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 5 @ 10.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 3.95 in, #5@ 6.13 in, #6@ 8.70 in, #7@ 11.87 in, #8@ 15.62 in, #9@ 19.7
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 7.29 in, #5@ 11.31 in, #6@ 16.05 in, #7@ 18 in, #8@

Min footing T&S reinf Area	3.08	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 7.41 in		#4@ 14.81 in
#5@ 11.48 in		#5@ 22.96 in
#6@ 16.30 in		#6@ 32.59 in

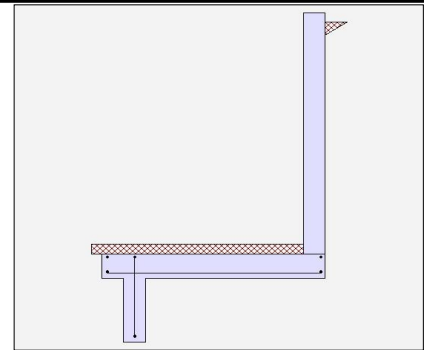
Cantilevered Retaining Wall

Criteria

Retained Height	=	12.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,666.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footings Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	108.0 #/ft
...Height to Top	=	12.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.25 Ratio < 1.5!
Sliding	=	1.17 Ratio < 1.5!
Total Bearing Load	=	4,806 lbs
...resultant ecc.	=	45.69 in
Soil Pressure @ Toe	=	2,432 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,666 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	3,405 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	27.4 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	4,807.3 lbs
less 100% Passive Force	= -	3,726.0 lbs
less 100% Friction Force	= -	1,922.3 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	1,562.6 lbs NG

Vertical component of active lateral soil pressure IS
NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	12.00
Rebar Size	=	# 7
Rebar Spacing	=	9.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.957
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	6,580.8

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	29,692.8
Moment....Allowable	=	31,026.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	57.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	9.56

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	150.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

	Vertical Reinforcing	Horizontal Reinforcing
Bottom Stem		
As (based on applied moment) :	0.7138 in ² /ft	
(4/3) * As :	0.9518 in ² /ft	Min Stem T&S Reinf Area 3.600 in ²
200bd/fy : 200(12)(9.5625)/60000 :	0.3825 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.288 in ² /ft
0.0018bh : 0.0018(12)(12) :	0.2592 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.7138 in ² /ft	#4@ 8.33 in #4@ 16.67 in
Provided Area :	0.8 in ² /ft	#5@ 12.92 in #5@ 25.83 in
Maximum Area :	1.2954 in ² /ft	#6@ 18.33 in #6@ 36.67 in

Footing Data

Toe Width	=	9.25 ft
Heel Width	=	1.00
Total Footing Width	=	10.25
Footing Thickness	=	15.00 in
Key Width	=	12.00 in
Key Depth	=	40.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 3,405	0 psf
Mu' : Upward	= 640,453	0 ft-#
Mu' : Downward	= 174,291	0 ft-#
Mu: Design	= 38,847	0 ft-#
Actual 1-Way Shear	= 27.36	0.03 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 7 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 5 @ 8.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 3.10 in, #5@ 4.81 in, #6@ 6.83 in, #7@ 9.32 in, #8@ 12.27 in, #9@ 15.54
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 5.71 in, #5@ 8.85 in, #6@ 12.56 in, #7@ 17.13 in, #

Min footing T&S reinf Area	3.32	in ²
Min footing T&S reinf Area per foot	0.32	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 7.41 in		#4@ 14.81 in
#5@ 11.48 in		#5@ 22.96 in
#6@ 16.30 in		#6@ 32.59 in

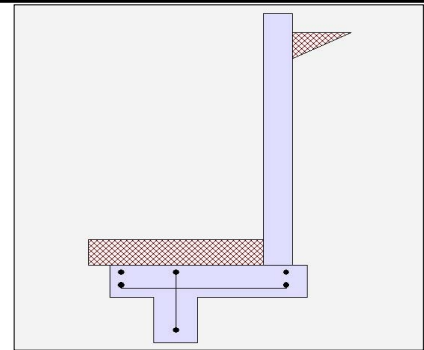
Cantilevered Retaining Wall

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footings Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.55 OK
Sliding	=	1.78 OK
Total Bearing Load	=	1,864 lbs
...resultant ecc.	=	5.79 in
Soil Pressure @ Toe	=	681 psf OK
Soil Pressure @ Heel	=	148 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	953 psf
ACI Factored @ Heel	=	207 psf
Footing Shear @ Toe	=	15.5 psi OK
Footing Shear @ Heel	=	3.0 psi OK
Allowable	=	75.0 psi
Sliding Calcs		
Lateral Sliding Force	=	933.9 lbs
less 100% Passive Force	= -	916.7 lbs
less 100% Friction Force	= -	745.7 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	10.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.379
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,224.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,448.0
Moment....Allowable	=	6,444.1

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	16.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0917 in ² /ft		
(4/3) * As :	0.1223 in ² /ft	Min Stem T&S Reinf Area 1.248 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.24 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	3.50 ft
Heel Width	=	1.00
Total Footing Width	=	4.50
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	14.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 953	207 psf
Mu' : Upward	= 55,828	13 ft-#
Mu' : Downward	= 20,409	61 ft-#
Mu: Design	= 2,952	49 ft-#
Actual 1-Way Shear	= 15.48	3.00 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	0.97	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	933.9	2.28	2,127.2	Soil Over HL (ab. water tbl)	220.0	4.33	953.3
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.33	953.3
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	256.7	1.75	449.2
				Surcharge Over Toe =			
				Stem Weight(s) =	650.0	3.83	2,491.7
				Earth @ Stem Transitions =			
Total	= 933.9	O.T.M. =	2,127.2	Footing Weight =	562.5	2.25	1,265.6
				Key Weight =	175.0	1.50	262.5
				Vert. Component =			
				Total =	1,864.2 lbs	R.M.=	5,422.3

Resisting/Overturning Ratio = 2.55
 Vertical Loads used for Soil Pressure = 1,864.2 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.027 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

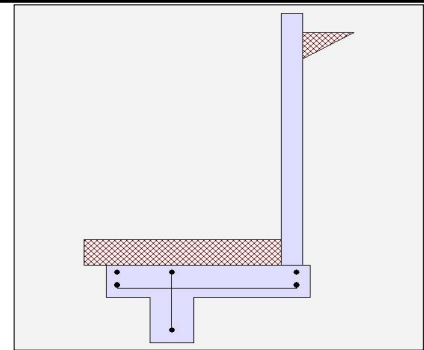
Cantilevered Retaining Wall

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.25 OK
Sliding	=	1.69 OK
Total Bearing Load	=	1,649 lbs
...resultant ecc.	=	8.65 in
Soil Pressure @ Toe	=	681 psf OK
Soil Pressure @ Heel	=	26 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	953 psf
ACI Factored @ Heel	=	36 psf
Footing Shear @ Toe	=	13.9 psi OK
Footing Shear @ Heel	=	1.8 psi OK
Allowable	=	75.0 psi
Sliding Calcs		
Lateral Sliding Force	=	933.9 lbs
less 100% Passive Force	= -	916.7 lbs
less 100% Friction Force	= -	659.7 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	9.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.518
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,224.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,448.0
Moment....Allowable	=	4,722.4

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	24.0
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1384 in ² /ft		
(4/3) * As :	0.1845 in ² /ft	Min Stem T&S Reinf Area 0.936 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.17 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.2667 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	0.67
Total Footing Width	=	4.67
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	14.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 953	36 psf
Mu' : Upward	= 66,364	1 ft-#
Mu' : Downward	= 26,656	15 ft-#
Mu: Design	= 3,309	15 ft-#
Actual 1-Way Shear	= 13.91	1.82 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 11.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.26 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	1.01	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	933.9	2.28	2,127.2	Soil Over HL (ab. water tbl)	110.0	4.58	504.2
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.58	504.2
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	293.3	2.00	586.7
=				Surcharge Over Toe =			
Total	933.9	O.T.M.	2,127.2	Stem Weight(s) =	487.5	4.25	2,071.9
				Earth @ Stem Transitions =			
				Footing Weight =	583.3	2.33	1,361.1
				Key Weight =	175.0	1.50	262.5
				Vert. Component =			
				Total =	1,649.2 lbs	R.M.=	4,786.3

Resisting/Overturning Ratio = 2.25
 Vertical Loads used for Soil Pressure = 1,649.2 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.026 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

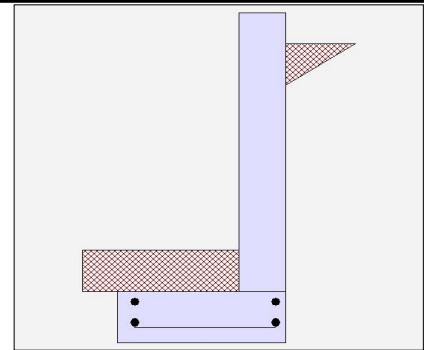
Cantilevered Retaining Wall

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	1.89 OK
Sliding	=	1.72 OK
Total Bearing Load	=	882 lbs
...resultant ecc.	=	5.45 in
Soil Pressure @ Toe	=	779 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,090 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	7.7 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi
Sliding Calcs		
Lateral Sliding Force	=	467.2 lbs
less 100% Passive Force	= -	450.0 lbs
less 100% Friction Force	= -	352.9 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.134
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	544.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	725.3
Moment....Allowable	=	5,412.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	7.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0272 in ² /ft		
(4/3) * As :	0.0362 in ² /ft	Min Stem T&S Reinf Area 0.864 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	1.75 ft
Heel Width	=	0.67
Total Footing Width	=	2.42
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	6.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,090	0 psf
Mu' : Upward	= 14,873	0 ft-#
Mu' : Downward	= 5,102	0 ft-#
Mu: Design	= 814	0 ft-#
Actual 1-Way Shear	= 7.74	0.03 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: No key defined

Min footing T&S reinf Area	0.52	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	467.2	1.61	752.7	Soil Over HL (ab. water tbl)	1.5	3.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)	2.42	3.5
Hydrostatic Force				Watre Table		
Buoyant Force =				Sloped Soil Over Heel =		
Surcharge over Heel =				Surcharge Over Heel =		
Surcharge Over Toe =				Adjacent Footing Load =		
Adjacent Footing Load =				Axial Dead Load on Stem =		
Added Lateral Load =				* Axial Live Load on Stem =		
Load @ Stem Above Soil =				Soil Over Toe =	128.3	112.3
				Surcharge Over Toe =		
				Stem Weight(s) =	450.0	937.5
				Earth @ Stem Transitions =		
Total	467.2	O.T.M. =	752.7	Footing Weight =	302.5	366.0
				Key Weight =		
				Vert. Component =		
				Total =	882.3 lbs	R.M.= 1,419.4

Resisting/Overturning Ratio = 1.89
 Vertical Loads used for Soil Pressure = 882.3 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.040 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

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Cantilevered Retaining Wall

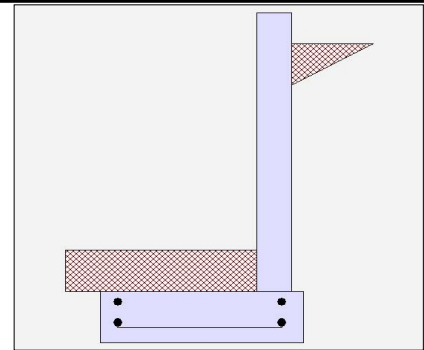
Code: IBC 2015,ACI 318-14,ACI 530-13

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	2.35 OK
Sliding	=	1.77 OK
Total Bearing Load	=	940 lbs
...resultant ecc.	=	4.53 in
Soil Pressure @ Toe	=	573 psf OK
Soil Pressure @ Heel	=	72 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	802 psf
ACI Factored @ Heel	=	101 psf
Footing Shear @ Toe	=	6.5 psi OK
Footing Shear @ Heel	=	1.2 psi OK
Allowable	=	75.0 psi
Sliding Calcs		
Lateral Sliding Force	=	467.2 lbs
less 100% Passive Force	= -	450.0 lbs
less 100% Friction Force	= -	376.2 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	6.00
Rebar Size	=	# 4
Rebar Spacing	=	18.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.295
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	544.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	725.3
Moment....Allowable	=	2,455.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	10.7
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in ² =	
Rebar Depth 'd'	in =	4.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	75.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS
NOT considered in the calculation of soil bearing**Load Factors**

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.041 in ² /ft		
(4/3) * As :	0.0547 in ² /ft	Min Stem T&S Reinf Area 0.648 in ²	
200bd/fy : 200(12)(4.25)/60000 :	0.17 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.144 in ² /ft	
0.0018bh : 0.0018(12)(6) :	0.1296 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1296 in ² /ft	#4@ 16.67 in	#4@ 33.33 in
Provided Area :	0.1333 in ² /ft	#5@ 25.83 in	#5@ 51.67 in
Maximum Area :	0.5757 in ² /ft	#6@ 36.67 in	#6@ 73.33 in

Footing Data

Toe Width	=	2.25 ft
Heel Width	=	0.67
Total Footing Width	=	2.92
Footing Thickness	=	10.00 in
Key Width	=	0.00 in
Key Depth	=	6.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 802	101 psf
Mu' : Upward	= 18,888	2 ft-#
Mu' : Downward	= 8,434	11 ft-#
Mu: Design	= 871	9 ft-#
Actual 1-Way Shear	= 6.51	1.16 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 11.11 in, #5@ 17.22 in, #6@ 24.44 in, #7@ 33.33 in, #8@ 43.88 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: No key defined

Min footing T&S reinf Area	0.63	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 11.11 in	#4@ 22.22 in	
#5@ 17.22 in	#5@ 34.44 in	
#6@ 24.44 in	#6@ 48.89 in	

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	467.2	1.61	752.7	Soil Over HL (ab. water tbl)	73.3	2.83	207.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.83	207.8
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	165.0	1.13	185.6
=				Surcharge Over Toe =			
Total	467.2	O.T.M.	752.7	Stem Weight(s) =	337.5	2.50	843.8
				Earth @ Stem Transitions =			
				Footing Weight =	364.6	1.46	531.7
				Key Weight =			
				Vert. Component =			
				Total =	940.4 lbs	R.M.=	1,768.8

Resisting/Overturning Ratio = 2.35
 Vertical Loads used for Soil Pressure = 940.4 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.025 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

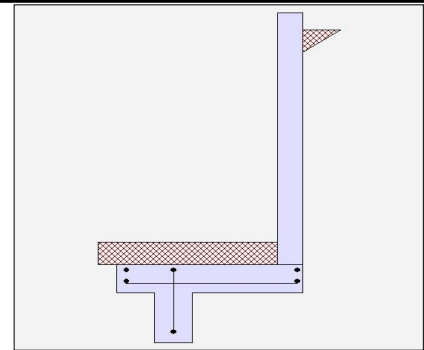
Cantilevered Retaining Wall

Criteria

Retained Height	=	7.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	8.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.86 OK
Sliding	=	1.60 OK
Total Bearing Load	=	1,904 lbs
...resultant ecc.	=	12.14 in
Soil Pressure @ Toe	=	876 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,227 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	19.2 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi
Sliding Calcs		
Lateral Sliding Force	=	1,227.2 lbs
less 100% Passive Force	= -	1,200.0 lbs
less 100% Friction Force	= -	761.7 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge
Design Data		
fb/FB + fa/Fa	=	0.488
Total Force @ Section		
Service Level	lbs =	
Strength Level	lbs =	1,666.0
Moment....Actual		
Service Level	ft-# =	
Strength Level	ft-# =	3,887.3
Moment....Allowable	=	7,959.6
Shear.....Actual		
Service Level	psi =	
Strength Level	psi =	22.2
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1456 in ² /ft		
(4/3) * As :	0.1942 in ² /ft	Min Stem T&S Reinf Area 1.440 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1942 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.3 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	4.25 ft
Heel Width	=	0.67
Total Footing Width	=	4.92
Footing Thickness	=	10.00 in
Key Width	=	12.00 in
Key Depth	=	18.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,227	0 psf
Mu' : Upward	= 89,622	0 ft-#
Mu' : Downward	= 30,092	0 ft-#
Mu: Design	= 4,961	0 ft-#
Actual 1-Way Shear	= 19.20	0.04 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 4 @ 9.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 10.09 in, #5@ 15.64 in, #6@ 22.20 in, #7@ 30.27 in, #8@ 39.86 in, #9@ 5
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	1.06	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	1,227.2	2.61	3,204.4	Soil Over HL (ab. water tbl)	2.6	12.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)	4.92	12.6
Hydrostatic Force				Watre Table		
Buoyant Force =				Sloped Soil Over Heel =		
Surcharge over Heel =				Surcharge Over Heel =		
Surcharge Over Toe =				Adjacent Footing Load =		
Adjacent Footing Load =				Axial Dead Load on Stem =		
Added Lateral Load =				* Axial Live Load on Stem =		
Load @ Stem Above Soil =				Soil Over Toe =	311.7	662.3
				Surcharge Over Toe =		
				Stem Weight(s) =	750.0	3,437.5
				Earth @ Stem Transitions =		
Total	= 1,227.2	O.T.M. =	3,204.4	Footing Weight =	615.0	1,512.9
				Key Weight =	225.0	337.5
				Vert. Component =		
Resisting/Overturning Ratio		= 1.86		Total =	1,904.2 lbs	R.M.= 5,962.8
Vertical Loads used for Soil Pressure =		1,904.2 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.		

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.037 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

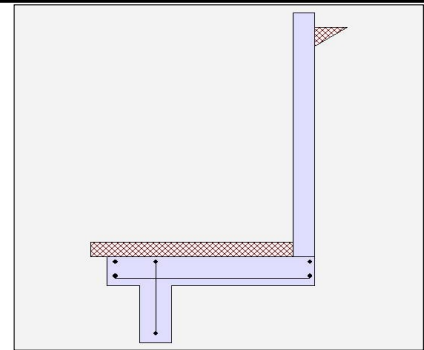
Cantilevered Retaining Wall

Criteria

Retained Height	=	8.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footing Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.98 OK
Sliding	=	1.64 OK
Total Bearing Load	=	2,432 lbs
...resultant ecc.	=	14.94 in
Soil Pressure @ Toe	=	825 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	1,155 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	17.7 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,620.0 lbs
less 100% Passive Force	= -	1,687.5 lbs
less 100% Friction Force	= -	972.9 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	10.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.602
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,176.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	5,802.7
Moment....Allowable	=	9,623.1

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	29.3
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.2197 in ² /ft		
(4/3) * As :	0.293 in ² /ft	Min Stem T&S Reinf Area 1.632 in ²	
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.2475 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.372 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8382 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	5.75 ft
Heel Width	=	0.67
Total Footing Width	=	6.42
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	24.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	<u>Toe</u>	<u>Heel</u>
Factored Pressure	= 1,155	0 psf
Mu' : Upward	= 154,644	0 ft-#
Mu' : Downward	= 56,934	0 ft-#
Mu: Design	= 8,143	0 ft-#
Actual 1-Way Shear	= 17.68	0.04 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 10.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 8.13 in, #5@ 12.61 in, #6@ 17.90 in, #7@ 24.41 in, #8@ 32.14 in, #9@ 40
 Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
 Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	1.66	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 9.26 in		#4@ 18.52 in
#5@ 14.35 in		#5@ 28.70 in
#6@ 20.37 in		#6@ 40.74 in

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Cantilevered Retaining Wall

Code: IBC 2015,ACI 318-14,ACI 530-13

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	1,620.0	3.00	4,860.0	Soil Over HL (ab. water tbl)	2.9	18.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)	6.42	18.8
Hydrostatic Force				Watre Table		
Buoyant Force =				Sloped Soil Over Heel =		
Surcharge over Heel =				Surcharge Over Heel =		
Surcharge Over Toe =				Adjacent Footing Load =		
Adjacent Footing Load =				Axial Dead Load on Stem =		
Added Lateral Load =				* Axial Live Load on Stem =		
Load @ Stem Above Soil =				Soil Over Toe =	316.3	909.2
				Surcharge Over Toe =		
				Stem Weight(s) =	850.0	5,170.8
				Earth @ Stem Transitions =		
Total	= 1,620.0	O.T.M. =	4,860.0	Footing Weight =	963.0	3,091.2
				Key Weight =	300.0	450.0
				Vert. Component =		
				Total =	2,432.2 lbs	R.M.= 9,640.1

Resisting/Overturning Ratio = 1.98
 Vertical Loads used for Soil Pressure = 2,432.2 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.030 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

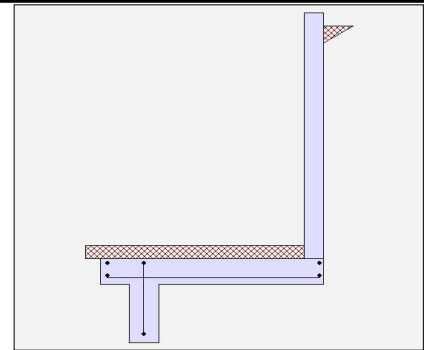
Cantilevered Retaining Wall

Criteria

Retained Height	=	9.00 ft
Wall height above soil	=	0.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	40.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	110.00 pcf
Footings Soil Friction	=	0.400
Soil height to ignore for passive pressure	=	12.00 in

**Surcharge Loads**

Surcharge Over Heel	=	0.0 psf
NOT Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
NOT Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type		Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary**Wall Stability Ratios**

Overturning	=	1.99 OK
Sliding	=	1.54 OK

Total Bearing Load	=	2,826 lbs
...resultant ecc.	=	18.03 in

Soil Pressure @ Toe	=	808 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	1,131 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	19.7 psi OK
Footing Shear @ Heel	=	0.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,000.0 lbs
less 100% Passive Force	= -	1,959.4 lbs
less 100% Friction Force	= -	1,130.5 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2015,ACI
Dead Load	1.400
Live Load	1.700
Earth, H	1.700
Wind, W	1.300
Seismic, E	1.000

Stem Construction

Design Height Above Ftg	ft =	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	LRFD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	8.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.700
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,754.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	8,262.0
Moment....Allowable	=	11,799.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	37.1
Shear.....Allowable	psi =	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	100.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

Stem OK

Concrete Stem Rebar Area Details

	Vertical Reinforcing	Horizontal Reinforcing
Bottom Stem		
As (based on applied moment) :	0.3128 in ² /ft	
(4/3) * As :	0.4171 in ² /ft	Min Stem T&S Reinf Area 1.824 in ²
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.3128 in ² /ft	#4@ 12.50 in #4@ 25.00 in
Provided Area :	0.465 in ² /ft	#5@ 19.38 in #5@ 38.75 in
Maximum Area :	0.8382 in ² /ft	#6@ 27.50 in #6@ 55.00 in

Footing Data

Toe Width	=	7.00 ft
Heel Width	=	0.67
Total Footing Width	=	7.67
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	27.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,131	0 psf
Mu' : Upward	= 221,640	0 ft-#
Mu' : Downward	= 84,378	0 ft-#
Mu: Design	= 11,439	0 ft-#
Actual 1-Way Shear	= 19.68	0.04 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 8.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 9.00 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 7.05 in, #5@ 10.94 in, #6@ 15.52 in, #7@ 21.17 in, #8@ 27.88 in, #9@ 35
Heel: phiMn = phi'5'lambda'sqrt(fc)'Sm
Key: #4@ 9.25 in, #5@ 14.35 in, #6@ 18 in, #7@ 18 in, #8@ 18

Min footing T&S reinf Area	1.99	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 9.26 in		#4@ 18.52 in
#5@ 14.35 in		#5@ 28.70 in
#6@ 20.37 in		#6@ 40.74 in

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	2,000.0	3.33	6,666.7	Soil Over HL (ab. water tbl)	3.3	7.67	25.3
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		7.67	25.3
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =	385.0	3.50	1,347.5
				Surcharge Over Toe =			
				Stem Weight(s) =	950.0	7.33	6,966.7
				Earth @ Stem Transitions =			
Total	= 2,000.0	O.T.M. =	6,666.7	Footing Weight =	1,150.5	3.84	4,412.2
				Key Weight =	337.5	1.50	506.3
				Vert. Component =			
				Total =	2,826.3 lbs	R.M.=	13,257.9

Resisting/Overturning Ratio = 1.99
 Vertical Loads used for Soil Pressure = 2,826.3 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.028 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.